



DEPARTMENT OF
HOUSING AND HUMAN CONCERNS
COUNTY OF MAUI

RECEIVED

ALAN M. ARAKAWA
Mayor

CAROL K. REIMANN
Director

JAN SHISHIDO
Deputy Director

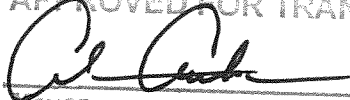
2017 JUL 10 PM 3:19
2200 MAIN STREET • SUITE 546 • WAILUKU, HAWAII 96793 • PHONE (808) 270-7805 • FAX (808) 270-7165
MAILING ADDRESS: 200 SOUTH HIGH STREET • WAILUKU, HAWAII 96793 • EMAIL: director.hhc@mauicounty.gov

July 10, 2017

Honorable Alan M. Arakawa
Mayor, County of Maui
200 South High Street
Wailuku, Hawaii 96793

For Transmittal to:

Honorable Chair Stacy Crivello
Housing, Human Services,
and Transportation Committee
Maui County Council
200 South High Street
Wailuku, Hawaii 96793

APPROVED FOR TRANSMITTAL

Mayor Date 7/10/17

Dear Chair Crivello:

**SUBJECT: COUNTY-OWNED PARCEL ON NORTH PAPA AVENUE
(KAHULUI) (HHT-14)**

Thank you for your correspondence of June 20, 2017 pertaining to your committee's discussion on the above subject. Your committee requested the Department's assistance in ensuring the scope of the feasibility study includes the items listed below in bold which are followed by the Department's responses.

1. **A copy of the renovation plans UHMC has prepared for the three dorm structures and recreation center on property.**

Per your request, please see attached plans developed for UHMC to transform the old dormitories into a Hospitality Training Academy.

2. **The best use of the property, including the feasibility and practicality of establishing a mixed-use project involving affordable rental units and a homeless facility.**

The best use of the property will be determined after a structural engineer has evaluated the soundness of the existing buildings. The buildings have sat vacant and unused for a length of time and, therefore, a full structural assessment would be required to address potential safety concerns. The

plan is to first conduct a forensic study of the existing structures, and then hire a consultant to move forward, depending upon the forensic results. The department is currently preparing the RFP to hire a structural engineer.

3. Whether existing structures on the property are sound and should be renovated, or whether they should be demolished.

Please see response to question 2.

4. Any useful accessories such as a bus stop to the project.

There is currently a bus stop located at Roselani Place, which is across the street from the parcel.

5. A projected timeframe for completion.

Until we can assess the viability of the existing structures, coming up with a timeframe is premature.

In response to your request for confirmation on whether UHMC intends to remain on the lease agreement until the year 2035, we have confirmed that UHMC has decided to move their training academy to a location on campus and has no intention of continuing the lease. The Department is prepared to terminate with UHMC once we determine if we can salvage the existing structures. If the buildings need to be demolished, then UHMC will pay for the demolition; after which, the department will terminate the lease.

Thank you for the opportunity to provide this information. Should you have any questions, please feel free to contact me at Ext. 7805.

Sincerely,



CAROL K. REIMANN
Director of Housing and Human Concerns

Attachment

GENERAL NOTES

- THE CONTRACTOR SHALL VERIFY ALL FIELD DIMENSIONS AND CONDITIONS PRIOR TO STARTING WORK. ALL DISCREPANCIES SHALL BE PROMPTLY REPORTED TO THE ENGINEER.
- ALL OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT OR STRUCTURAL ENGINEER BEFORE PROCEEDING WITH ANY WORK SO INVOLVED.
- ALL WORK SHALL CONFORM TO THE INTERNATIONAL BUILDING CODE (IBC), LATEST EDITION, AS ADOPTED BY THE STATE OF HAWAII.
- UNLESS SPECIFICALLY DETAILED ELSEWHERE, CONTRACTOR SHALL FOLLOW TYPICAL DETAILS ON SHEETS 50-01 THROUGH 50-03.
- WHERE CONDITIONS HAVE NOT BEEN SPECIFICALLY INDICATED BUT ARE SIMILAR TO DETAILS SHOWN ON SHEETS 50-01 THROUGH 50-03, MODIFY TYPICAL DETAILS TO MEET SPECIAL CONDITIONS.
- STRUCTURAL DRAWINGS WHERE INDICATED HAVE BEEN DRAWN TO APPROXIMATE SCALE. UNLESS DRAWING SPECIFICALLY PROVIDES A DIMENSION, THE CONTRACTOR SHALL NOT USE OR SCALE STRUCTURAL DRAWINGS TO DETERMINE DIMENSIONS.
- PRIOR TO ANY DEMOLITION AND DURING CONSTRUCTION, THE EXISTING STRUCTURES SHALL BE SHORED AS REQUIRED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING. THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION DRAWINGS AND SHORING FOR ALL STRUCTURAL MEMBERS AS REQUIRED FOR STRUCTURAL STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION.

REINFORCING STEEL

- UNLESS OTHERWISE INDICATED ON PLANS AND/OR SCHEDULE, ALL REINFORCING STEEL SHALL BE HIGH STRENGTH GRADE REINFORCING BARS WHICH SHALL CONFORM TO THE STANDARD SPECIFICATION OF ASTM A630 GRADE 60, ALL REINFORCING STEEL TO BE WELDED SHALL CONFORM TO ASTM A706, GRADE 60. FOR SPECIAL CHORD REINFORCEMENT ASTM A15 GRADE 75 BARS SHALL BE USED AS SPECIFIED ON PLANS.
- LOW HYDROGEN WELDING RODS SHALL BE USED FOR ALL WELDING TO REINFORCING BARS.
- REINFORCING SHALL BE SPLICED ONLY AS SHOWN OR NOTED. SPLICES AT OTHER LOCATIONS SHALL BE MADE AWAY FROM POINTS OF MAXIMUM STRESS AND APPROVED BY THE ENGINEER. SEE DETAILS FOR "LAP SPLICES".
- BARS NOTED AS "CONT. VERTICAL AND HORIZONTAL WALL REINFORCING AND COLUMN REINFORCING SHALL HAVE A MINIMUM SPACE EQUAL TO 48 BAR DIAMETERS BUT NOT LESS THAN 24 INCHES.
- WHERE MECHANICAL SPLICES ARE REQUIRED, SPLICES SHALL BE ABLE TO DEVELOP 125% OF THE YIELD STRENGTH OF THE BAR AND SHALL BE STAGGERED 3'-0" BETWEEN ADJACENT BARS.
- PROVIDE DONNELS IN FOOTINGS THE SAME SIZE AND NUMBER AND IN THE SAME LOCATION AS VERTICAL COLUMN REINFORCING. DONNELS SHALL HAVE A MINIMUM PROJECTION EQUAL TO 48 BAR DIAMETERS BUT NOT LESS THAN 24 INCHES.
- ALL REINFORCING STEEL, ANCHOR BOLTS AND OTHER INSERTS SHALL BE SECURED IN POSITION PRIOR TO PLACING CONCRETE.
- MINIMUM CONCRETE COVER:
 - MINIMUM COVER FOR CAST-IN-PLACE CONCRETE (NON-PRESTRESSED):
 - FOOTING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:

	COVER (IN)
(1) CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3"
(2) FORMED CONCRETE SURFACES EXPOSED TO EARTH OR WEATHER:	
NO. 6 THROUGH NO. 18 BARS	2"
NO. 5 BAR, NO. 3 OR 0.31 WIRE, AND SMALLER	1 1/2"
(3) CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:	
SLABS, WALL, JOISTS:	
NO. 14 AND NO. 18 BARS	1 1/2"
NO. 11 BAR AND SMALLER	1"

BEAMS, COLUMNS:	
PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS	1 1/2"

C) CONCRETE COVER TOLERANCE SHALL BE AS FOLLOWS:

- FORMED SOFFITS OR MEMBERS 4" OR LESS IN THICKNESS: $\pm \frac{1}{8}"$
- MEMBERS 12" THICK OR LESS: $\pm \frac{3}{16}"$
- MEMBERS GREATER THAN 12" THICK: $\pm \frac{1}{4}"$

FOR ALL CASES, THE COVER SHALL NOT BE LESS THAN 1/2" OF WHAT IS INDICATED AND REQUIRED BY THE DRAWINGS AND SPECIFICATIONS.

- WHEN THE IBC REQUIRES A THICKNESS OF COVER FOR FIRE PROTECTION GREATER THAN THE MINIMUM CONCRETE STATED ABOVE, SUCH GREATER THICKNESS SHALL BE USED.

CONCRETE BLOCK MASONRY

- ALL MASONRY WORK SHALL CONFORM TO ACI 318-05.
- CONCRETE BLOCK SHALL BE TYPE II HOLLOW LOAD BEARING UNITS.
- MASONRY MORTAR SHALL BE TYPE "M", WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2,500 PSI CONFORMING TO ASTM C270.
- MASONRY GROUT SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2,500 PSI.
- ALL CELLS SHALL BE GROUTED SOLID. GROUT IN LIFTS NOT TO EXCEED 6'-0".
- UNLESS NOTED OTHERWISE, ALL WALLS SHALL BE CONSTRUCTED IN CONVENTIONAL RUNNING BOND.
- AT CMU WALLS PROVIDE HORIZONTAL JOINT REINFORCING AT 16" O.C.
- WIRE POSTIONERS - CMU WALL REINFORCING SHALL BE SECURED AGAINST DISPLACEMENT BY GALVANIZED NO. 9 GAUGE WIRE POSTIONERS SPACED AT NOT MORE THAN 200 TIMES THE BAR DIAMETER.

CONCRETE

- ALL CONCRETE WORK SHALL CONFORM TO ACI 318-05.
- SLEEVES EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS SHALL NOT BE PLACED IN STRUCTURAL CONCRETE UNLESS SPECIFICALLY DETAILED. PIPE MAY PASS THROUGH STRUCTURAL CONCRETE IN SLEEVES BUT SHALL NOT BE EMBEDDED THEREIN.
- SECURE ALL BOLTS, ANCHORS, INSERTS, ETC. AND VERIFY ALL GROOVES, SLOTS, AND FINISHES PRIOR TO PLACING CONCRETE.
- VERIFY LOCATIONS AND DIMENSIONS OF SLOTS, ANCHORS, DUCTS, ETC. RELATING TO MECHANICAL, ELECTRICAL, AND ARCHITECTURAL WORK BEFORE POURING CONCRETE.
- ALL CONCRETE WORK SHALL BE THOROUGHLY CONSOLIDATED DURING PLACEMENT USING A MECHANICAL VIBRATOR, BACKUP (EXTRA) VIBRATOR(S) SHALL BE PRESENT AT EACH FOR IN CASE OF FAILURE.
- ALL CONCRETE SHALL BE CURED FOR A PERIOD OF NOT LESS THAN 7 DAYS.
- LOCATIONS OF ALL CONSTRUCTION OR COLD JOINTS SHALL BE COORDINATED WITH AND APPROVED BY THE ENGINEER.
- 48 HOURS PRIOR TO THE POURING OF ANY STRUCTURAL CONCRETE, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO AN INSPECTION CAN BE MADE OF ALL FORMS AND REINFORCING STEEL.
- THE 28-DAY COMPRESSIVE STRENGTH OF CONCRETE SHALL BE AS FOLLOWS:

	STRENGTH (PSI)	MAX. AGGREGATE	W/C RATIO
BUILDING SLAB ON GRADE, EXTERIOR WALKWAY AND LANDINGS	4,500	1 1/2"	0.45
FOOTINGS AND GRADE BEAMS	4,500	1 1/2"	0.45

EPOXIED ANCHORS:

- ADHESIVE SHALL BE AN INJECTABLE TWO-COMPONENT EPOXY SUCH AS SPMSPON SET-UP, HILTI HIT-RE-500 SD OR APPROVED EQUAL.
- INSTALLATION OF EPOXIED ANCHORS SHALL FOLLOW ALL MANUFACTURER'S RECOMMENDATIONS.
- THREADED ANCHOR RODS SHALL BE HOT DIP GALVANIZED AND CONFORM TO ASTM A-193, GRADE B7. THREADED ANCHOR RODS SHALL BE UNC PLAIN THREADED, CLEAN, STRAIGHT AND FREE OF INDENTATIONS OR OTHER DEFECTS ALONG THEIR LENGTH.
- HOLES SHALL BE DRILLED WITH A CARBIDE-TIPPED DRILL BIT. IF HOLES ARE CORE DRILLED WITH DIAMOND-CORE BITS, EPOXY ADHESIVE SELECTED FOR USE BY THE CONTRACTOR SHALL BE SUITABLE FOR THIS APPLICATION WITH NO REDUCTION IN PUBLISHED ALLOWABLE SHEAR AND TENSION LOADS.
- DIAMETER OF HOLES SHALL BE EQUAL TO THREADED ROD DIAMETER + 1/8".
- ANCHORS SHALL BE INSTALLED IN CONCRETE/CMU THAT HAS REACHED ITS FULL DESIGN COMPRESSIVE STRENGTH.
- INSTALLATION OF EPOXIED ANCHORS SHALL BE PERFORMED UNDER THE SUPERVISION OF A SPECIAL INSPECTOR IN ACCORDANCE WITH SECTIONS 1704.4 AND 1704.13 OF THE IBC. THE SPECIAL INSPECTOR MUST BE ON THE JOBSITE TO VERIFY ANCHOR TYPE, ANCHOR DIMENSIONS, CONCRETE TYPE, CONCRETE/CMU COMPRESSIVE STRENGTH, HOLE DIMENSIONS, HOLE CLEANING PROCEDURES, ANCHOR SPACING, EDGE DISTANCES, ANCHOR EMBEDMENT, AND TIGHTENING TORQUE.

DESIGN LOADS

- SLOPED ROOF LIVE LOAD: 20 PSF
- FLOOR LIVE LOADS:
 - RESIDENTIAL: 40 PSF
 - STAIRS: 100 PSF
- WIND 105 MPH PER IBC, EXPOSURE C, $iw=1.0$, $Kzt=1.6$
- SEISMIC PER IBC, $Ss=0.98$, $S1=0.252$, $I_e=1.0$, SITE CLASS D
- ALL COMPONENTS TO BUILDING SHALL BE SECURED PROPERLY TO RESIST LATERAL SEISMIC FORCES IN ACCORDANCE TO THE IBC. DESIGN OF ATTACHMENTS FOR COMPONENTS SHALL BE THE RESPONSIBILITY OF SUB-CONTRACTOR INSTALLING SUCH COMPONENT

SPECIAL INSPECTION REQUIREMENTS LIST BELOW

- SPECIAL INSPECTION REQUIREMENTS:
 - THE OWNER SHALL EMPLOY A SPECIAL INSPECTOR FOR WORK REQUIRING SPECIAL INSPECTION PER CHAPTER 17 OF THE 2006 IBC AS ADOPTED BY THE COUNTY OF MAUI.
 - SPECIAL INSPECTION FIRM SHALL HAVE AT LEAST 6 YEARS OF EXPERIENCE AND SHALL BE CERTIFIED WITHIN THE BUILDING DEPARTMENT JURISDICTION OF CONSTRUCTION (IF SUCH CERTIFICATION EXISTS).
 - THE CONTRACTOR SHALL COORDINATE HIS WORK WITH THE SPECIAL INSPECTOR TO ENSURE THAT ALL ELEMENTS REQUIRING SPECIAL INSPECTION ARE INSPECTED.
 - SPECIAL INSPECTION IS REQUIRED FOR THE FOLLOWING TYPES OF WORK:
 - CONCRETE:
 - OBSERVE THE TAPPING OF TEST SPECIMENS AND THE PLACING OF CONCRETE FOR ALL ELEMENTS.
 - REINFORCING STEEL:
 - OBSERVE THE PLACEMENT OF ALL REINFORCING. THE SPECIAL INSPECTOR NEED NOT BE PRESENT CONTINUOUSLY DURING THE PLACING OF REINFORCING STEEL, PROVIDED HE HAS INSPECTED FOR CONFORMANCE WITH THE APPROVED PLANS PRIOR TO THE CLOSING OF FORMS FOR THE DELIVERY OF CONCRETE TO THE JOBSITE.
 - ANCHOR BOLTS FOR STEEL COLUMNS AND POST
 - EXPANSION BOLTS AND EPOXIED ANCHORS/REBAR
 - CONTINUOUS LOAD PATH AND SHEARWALL HOLD DOWNS

CONCRETE REPAIR/REHABILITATION

PROJECT CONDITIONS:

- DO NOT FORM MATERIAL IF IT IS RAINING OR IF RAIN IS IMMINENT.
- PRECAUTIONS SHOULD BE TAKEN TO AVOID DAMAGE TO ANY SURFACE NEAR THE WORK ZONE DUE TO SPILLAGE.
- LEAVE FINISHED WORK IN NEAT, CLEAN CONDITION WITH NO EVIDENCE OF SPILL OVER ONTO ADJACENT SURFACES.

PRODUCTS:

- ANTI-CORROSION PROTECTIVE COATING (CORROSION INHIBITOR): TWO-COMPONENT, ACRYLIC RESIN EPOXY SYSTEM FORMULATED TO INHIBIT THE CORROSION OF REINFORCING STEEL AND TO ACT AS A BONDING AGENT. SUCH AS: SIKKA ARMATAC 110 EPOXID, MASTER EMACOPIA24 OR APPROVED EQUAL.
- FAST SETTING, HIGH STRENGTH PATCHING MORTAR (POLYMER MODIFIED MORTAR) FOR HORIZONTAL APPLICATIONS: ACRYLIC POLYMER-MODIFIED, PORTLAND CEMENT BASED, TROWEL-GRADE PATCHING MORTAR. CONFORM TO REQUIREMENTS OF ASTM C 884, SUCH AS SKATOP 123 PLUS, MASTER EMACO R310 OR T430, OR APPROVED EQUAL.
- FAST SETTING, HIGH STRENGTH PATCHING MORTAR (POLYMER MODIFIED MORTAR) FOR VERTICAL APPLICATIONS: TWO-COMPONENT (OR SINGLE COMPONENT), ACRYLIC POLYMER-MODIFIED, PORTLAND CEMENT BASED, NON-SAG PATCHING MORTAR. CONFORM TO REQUIREMENTS OF ASTM C 884, SUCH AS SKATOP 123 PLUS, MASTER EMACO H426, OR APPROVED EQUAL.

PREPARATION - GENERAL:

- ACI 503R, AND 503.3.2, AND 503.4 FOR EPOXY RESIN SYSTEMS. MIX THE PATCHING MATERIALS WITH OR WITHOUT FILLERS IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS. ALL APPLICATIONS OF THE MIXED MATERIALS SHALL BE PERFORMED WITHIN THE WORKING LIFE OR POT LIFE OF THE PATCHING SYSTEM. UNUSED MIXED MATERIALS WHICH HAVE REACHED THE END OF THE WORKING OR POT LIFE SHALL BE REMOVED FROM THE JOB SITE AT THE CONTRACTOR'S EXPENSE. FIELD MIXING AND SIZE OF BATCH SHALL BE DETERMINED BY THE CONTRACTOR. PATCHING SYSTEMS SHALL BE PROVIDED AS INDICATED AND REQUIRED BY SPECIFICATIONS.
- CLEANING: PREPARE ALL SURFACES REQUIRING PATCHING SO THAT THEY WILL BE STRUCTURALLY SOUND, CLEAN, AND FREE OF DIRT, LOOSE CONCRETE, LOOSE MORTAR PARTICLES, PAINT, FILMS, PROTECTIVE COATINGS, EFFLORESCENCE, LANTANE, AND OTHER MATTER DETRIMENTAL TO PROPER ADHESION.
- CHECK AREAS AROUND CRACKS, BUSTERS, OLD PATCHES, AND RUST SPOTS BY TAPPING SURFACE WITH A HAMMER, OTHER BLUNT INSTRUMENT, OR USING A CHAIN, SOUND FOR LOOSE OR HOLLOW AREAS AND MARK AREAS FOR REMOVAL OF UNSOUND CONCRETE OR MORTAR.
- IF THE CONCRETE IS NOT IN A STATE OF CONCRETE DELAMINATION AS DETERMINED BY SOUNDING WITH A HAMMER OR CHAIN SHALL BE CLEANED OF ALL DIRT, LOOSE MORTAR PARTICLES, PAINT, FILMS, LANTANE AND ALL OTHER MORTAR DETRIMENTAL TO PROPER ADHESION.
- REMOVE UNSOUND MATERIALS AND FILMS BY MEANS OF CHIPPING, SAW CUTTING, BUSH HAMMERING, NEEDLE GUNNING, SAND BLASTING, OR OTHER SUITABLE METHOD.
- WASH DEBRIS FROM WORK SURFACES WITH CLEAN WATER. REMOVE ANY REMAINING EFFLORESCENCE BY AOD WASH, MURATIC OR SIMILAR, AND RINSE WITH CLEAN WATER.

PREPARATION - REINFORCEMENT:

- MECHANICALLY REMOVE ALL RUST AND LOOSE MATERIAL FROM ALL EXPOSED RUSTED REINFORCING STEEL, TO SOUND METAL BY POWER BRUSHING, NEEDLE SCALER, OR SAND BLASTING.
- APPLY ANTI-CORROSION PROTECTIVE COATING ACCORDING TO MANUFACTURER'S PRINTED INSTRUCTIONS TO ALL EXPOSED AND CLEANED REINFORCEMENT BARS AND CONCRETE SURFACES TO RECEIVE PATCH.
- USING BRUSH OR ROLLER, SCRUB PRIMER BONDING AGENT ONTO WORK SURFACES WHILE ALSO COMPLETELY COATING EXPOSED REINFORCING STEEL AND CONCRETE.
- DO NOT RESURFACE OVER HARDENED PRIMER BONDING AGENT. IF PRIMER BONDING AGENT BECOMES THICK, FREE, MECHANICALLY ROUGHEN SURFACE AND REPRIME.

SPALL REPAIR (INCLUDES CRACKED/CLOSED SPALL CORNER OF PEDESTAL):

- SAW CUT OR ROUT OUT SPALLS TO A MINIMUM WIDTH AND DEPTH OF 1/2 INCH.
- REFER TO GENERAL PREPARATION INSTRUCTIONS ABOVE.
- IF REINFORCING BARS IS EXPOSED, REFER TO REINFORCING PREPARATION INSTRUCTIONS ABOVE.
- MECHANICALLY SCRAFF WORK SURFACES TO OBTAIN AGGREGATE FRACTURE WITHIN A MINIMUM SURFACE PROFILE OF PLUS OR MINUS 1/8 INCH.
- APPLY PRIMER BONDING AGENT PER ABOVE INSTRUCTIONS AND MANUFACTURER'S PRINTED INSTRUCTIONS.
- WHILE PRIMER BONDING AGENT IS STILL TACKY, MIX AND APPLY PREPARED PATCHING MORTAR TO PREPARED SPALL FOLLOWING MANUFACTURER'S PRINTED INSTRUCTIONS.
- PLACE PATCHING MATERIAL IN THIN LAYERS AS RECOMMENDED BY THE MANUFACTURER. EACH INTERMEDIATE LAYER SHALL BE CROSS-SORTIFIED FOR MECHANICAL BONDING. ALL LAYERS FOR EACH PATCH SHALL BE PLACED ON THE SAME DAY. USE HAND TAMPERS TO CONSOLIDATE THE PATCHING MATERIAL. LEVEL EACH LAYER AND SMOOED THE FINAL SURFACE. REMOVE EXCESS PATCHING MATERIAL ON ADJACENT SURFACES BEFORE IT HARDENS. DO NOT FEATHER OUT ONTO ADJACENT SURFACES. UPON COMPLETION OF FINISHING OPERATIONS, APPLY EVAPORATION REDUCER AND THEN APPLY CURING COMPOUND IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. PROTECT PATCHES WITH WATERPROOF FORMS AND/OR COVERS UNTIL THE PATCHES ARE PROPERLY CURED.
- USE PROPER PATCHING MATERIAL DEPENDING ON IF THE LOCATION OF THE SPALL IS HORIZONTAL, VERTICAL, OR OVERHEAD.
- MINIMUM DEPTH OF SPALL PATCH OVER REINFORCING SHALL BE 1 INCH.
- DO NOT FEATHER EDGES OF SPALL PATCH. SAW CUT OR ROUT EDGE OF REPAIR TO OBTAIN A 1/2 INCH MINIMUM VERTICAL EDGE TO REPAIRED AREA.
- FINISHED PATCHED SPALL SHOULD MATCH EXISTING SURFACES IN TEXTURE AND APPEARANCE.
- AT EXPOSED AREAS, GRIND THE REPAIRED PATCH FLUSH WITH SURROUNDING SURFACES. PATCH TEXTURE TO MATCH EXISTING.

ABBREVIATIONS

A.B.	ANCHOR BOLT	H.S.B.	HIGH STRENGTH BOLT
ARCH.	ARCHITECTURAL OR ARCHITECT	INT.	INTERIOR
AT	BALANCE	JOINT	JOINT
BAL.	BOTTOM	KPSF	KIPS PER SQUARE FOOT
BOT.	BEAM	LONG.	LONGITUDINAL
B.W.	BASALTIC THERMITE BARRIER	MAX.	MAXIMUM
B.T.B.	CAST IN PLACE	M.B.	MACHINE BOLT
C.I.P.	CONSTRUCTION OR CONTROL JOINT	MECH.	MECHANICAL
C.L.	COMPLETE JOINT PENETRATION WELD	MIN.	MINIMUM
CLR	CENTER LINE	N.T.S.	NOT TO SCALE
CMU	CONCRETE MASONRY UNIT	NO.	NUMBER
COL.	COLUMN	O.C.	ON CENTER
CONC.	CONCRETE	OPP.	OPPOSITE
CONT.	CONTINUOUS	PL.	PLATE
CONN.	CONNECTION	PSF	POUNDS PER SQUARE INCH
CR.	CRACK	R.F. OR	REINFORCED OR
CS.	REBAR DIAMETER	REINFC.	REINFORCEMENT
DA.	DIAMETER	REQ'D.	REQUIRED
DIAG.	DIAGONAL	SHT.	SHEET
DIMS.	DIMENSIONS	SP.	SPACE OR SPACES
DET.	DETAIL	SMA.	SMALL
EA.	EACH	SQ.	SQUARE
E.F.	EXPANSION JOINT	STD.	STANDARD
ELEV.	ELEVATOR	T & B	TOP AND BOTTOM
EQ.	EQUAL	T.O.C.	TOP OF CONCRETE
EXP.	EXPANSION	T.O.F.	TOP OF FOOTING
EXT.	EXTERIOR	T.O.S.	TOP OF SLAB
E.W.	EACH WAY	T.O.W.	TOP OF WALL
FT.	FOOT OR FEET	TRANS.	TRANSVERSE
FTS.	FOOTING	TYPE	TYPICAL
GALV.	GALVANIZED	VERT.	VERTICAL
G.C.	GAGE	W/	WITH
GO.	GALVANIZE	W/O	WITH OUT
HOG OR H.O.G.	HOT DIPPED GALVANIZED	W.L.	WALL JOINT
HORIZ.	HORIZONTAL	W.W.F.	WELDED WIRE FABRIC
		W.W.M.	WELDED WIRE MESH

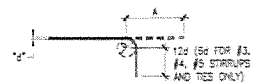


BAR BEND 135°



STANDARD HOOK 180°

BAR SIZE	D
#3, #4, #5	5d
#6, #7, #8	6d
#9, #10, #11	8d
#14, #18	10d

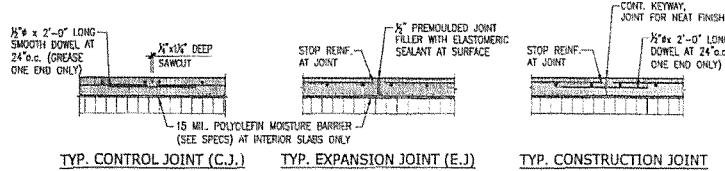


BAR BEND 90°

BAR SIZE	MINIMUM EXTENSION LENGTHS "A"			
	STANDARD HOOKS	TIES AND STIRRUPS	90° HOOKS	135° HOOKS
#3	5"	7"	5"	5"
#4	5"	7"	5"	5"
#5	5"	7"	5"	5"
#6	11"	13"	14"	12"
#7	13"	15"	16"	14"
#8	14"	17"	19"	16"

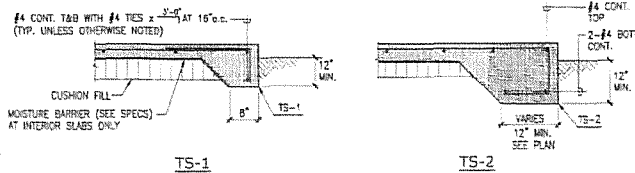
A TYPICAL REBAR BENDING

	UNIVERSITY OF HAWAII SCHOOL OF ENGINEERING UNIVERSITY OF HAWAII MAUI COLLEGE UH PROJECT NUMBER CC-11-5382 2248A & 2248B HOSPITALITY ACADEMY RENOVATION KAHULU, MAUI GENERAL NOTES AND TYPICAL DETAILS SLSH & ASSOC., INC. 1915 KAHULU STREET, 2ND FLOOR KAHULU, HAWAII 96701 DESIGNED BY: [Signature] CHECKED BY: [Signature] DATE: 10-30-2013 NO. 1000 SHEET 04 OF 04
--	--



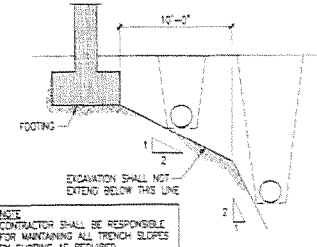
SAWCUT SHALL BE PERFORMED WITHIN 8 HOURS OF POUR AND AS SOON AS SLAB HAS SET AND IS FIRM ENOUGH SUCH THAT SAWCUT BLADE DOES NOT RIP, TEAR, OR DAMAGE SLAB OR UNRAVEL EDGES

A TYPICAL SLAB JOINT DETAILS
SD-01 NOT TO SCALE

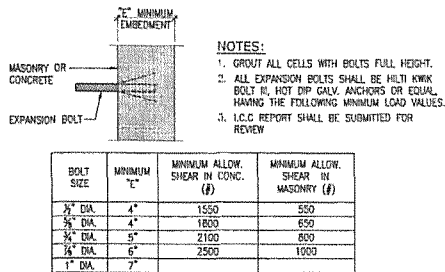


1. THIS DETAIL IS INTENDED TO SHOW DIMENSION OF THICKENED EDGE SLAB ONLY.
2. ALL SIDEWALK EDGES SHALL HAVE TS-1.

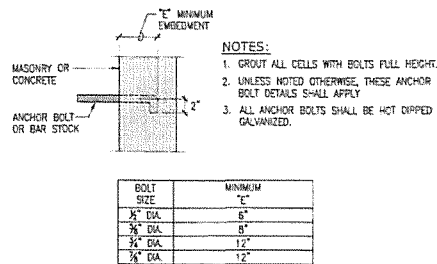
B THICKENED SLAB DETAIL (TS-1 and TS-2)
SD-01 NOT TO SCALE



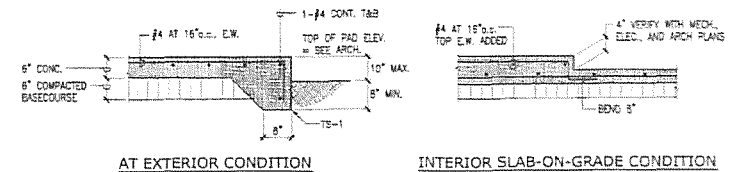
C TRENCH DETAIL
SD-01 NOT TO SCALE



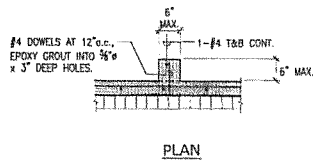
D EXPANSION BOLT
SD-01 NOT TO SCALE



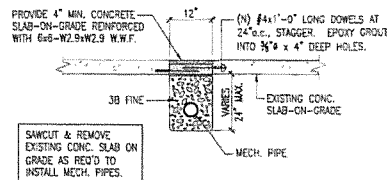
E ANCHOR BOLT
SD-01 NOT TO SCALE



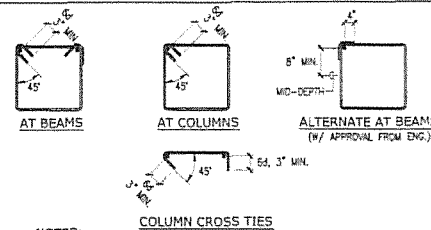
F TYPICAL RAISED PAD AT SLAB-ON-GRADE CONDITION
SD-01 NOT TO SCALE



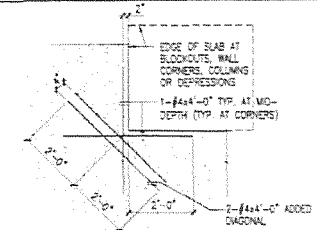
G TYP. CURB AT BASE OF STUD WALL
SD-01 NOT TO SCALE



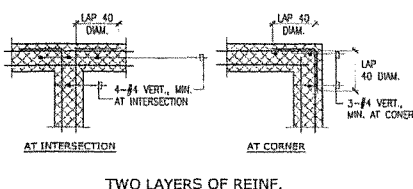
H SLAB ON GRADE REPAIR DETAIL
SD-01 NOT TO SCALE



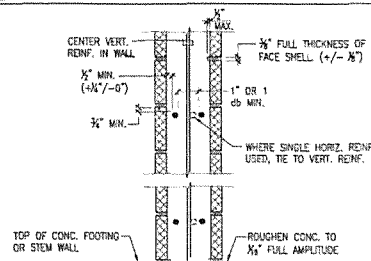
J TYPICAL STIRRUP AND TIE DETAIL
SD-01 NOT TO SCALE



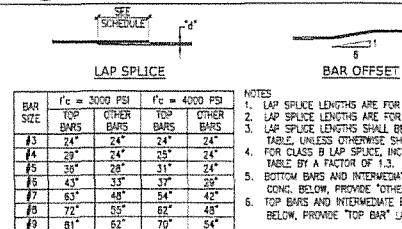
K TYP. S.O.G. RE-ENTRANT CORNER/BLOCKOUT DETAIL
SD-01 NOT TO SCALE



L TYP. CMU WALL BOND BEAM REINFORCING
SD-01 NOT TO SCALE



M TYPICAL CMU DETAIL
SD-01 NOT TO SCALE



N TYPICAL LAP SPLICE DETAIL
SD-01 NOT TO SCALE

REVISION	DATE	DESCRIPTION	BY	DATE

UNIVERSITY OF HAWAII
STATE OF HAWAII

UNIVERSITY OF HAWAII COLLEGE
144 PROJECT NUMBER CC-11-5382
2245A & 2245B HOSPITALITY ACADEMY RENOVATION
KAILUA, MAUI

TYPICAL DETAILS

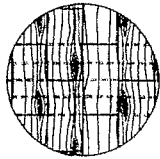
SLASH & ASSOC., INC.
1111 YOUNG STREET, 14TH FLOOR
HONOLULU, HAWAII 96813

PROJECT NO. CC-11-5382

ISSUED BY: SDN
CHECKED BY: RL
DATE: 5-30-2015

S0-01

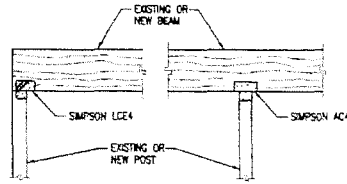
REV. 47 of 103 sheets



- NOTE**
1. LAY ALL PLYWOOD WITH FACE GRAIN ACROSS SUPPORTS.
 2. STAGGER ALL PANELS.
 3. UNLESS NOTED OTHERWISE, FOR SPECIAL SHEARWALL OR DIAPHRAGM LOCATIONS PROVIDE THE FOLLOWING MINIMUM NAILING:
A. 1/2" PLYWOOD - 8d AT 6" O.C. ALL FRAMING.
B. 5/8" PLYWOOD - 10d AT 6" O.C. ALL FRAMING.
C. 3/4" PLYWOOD - 10d AT 8" O.C. ALL FRAMING.
 4. PROVIDE SOLID BLOCKING AT ALL RIDGE, VALLEYS AND HIPS.

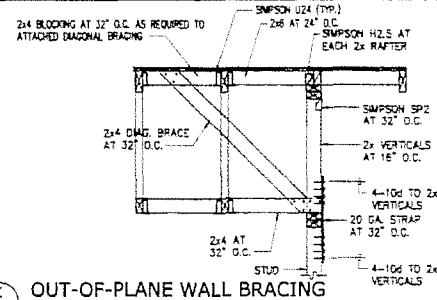
A TYP. PLYWOOD LAYOUT

50-02 NOT TO SCALE



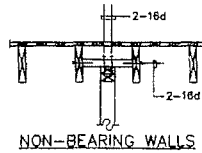
B TYP. BEAM TO POST CONNECTION

50-02 NOT TO SCALE

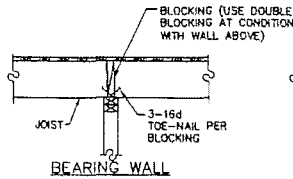


C OUT-OF-PLANE WALL BRACING

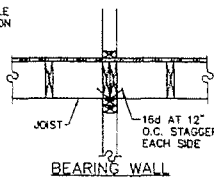
50-02 NOT TO SCALE



NON-BEARING WALLS



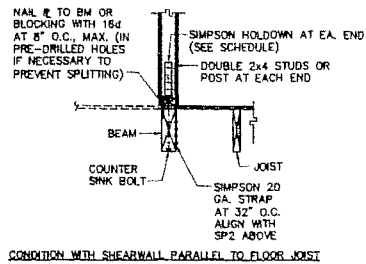
BEARING WALL



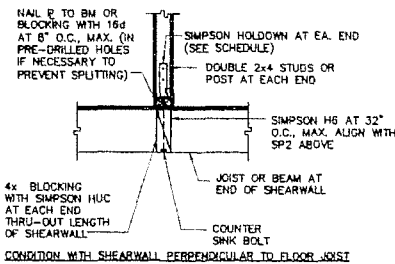
BEARING WALL

D TYP. STUDWALL (NON-SHEARWALL) TO FLOOR CONNECTION

50-02 NOT TO SCALE



CONDITION WITH SHEARWALL PARALLEL TO FLOOR JOIST



CONDITION WITH SHEARWALL PERPENDICULAR TO FLOOR JOIST

FOR SHEARWALLS ON 2nd FLR WHICH DOES NOT CONTINUE TO 1st FLOOR

G DETAIL OF SHEARWALL END POST/DBL STD & NAILING REQ'D AT 2ND FLOOR

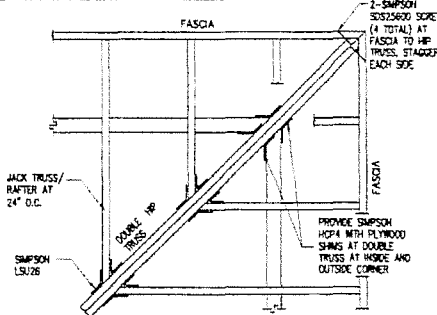
50-02 NOT TO SCALE

WOOD:

1. ALL WOOD STRUCTURAL MEMBERS SHALL BE DOUGLAS FIR AND SHALL MEET THE FOLLOWING MINIMUM GRADES AS ESTABLISHED BY THE WEST COAST LUMBER INSPECTION BUREAU:
BEAMS NO. 1
JOISTS/WATERS/TRUSS NO. 1
POSTS AND STUDS NO. 1
BLOCKING, PLATES, LEDGERS ETC. NO. 2
2. PROVIDE 30# FELT BELOW ALL PLATES RESTING ON CONCRETE OR MASONRY.
3. PROVIDE STANDARD WASHERS AT ALL BOLTS AND NUTS BEARING ON WOOD.
4. HOLES THRU PLATES AND STUDS SHALL BE CENTERED IN THE MEMBER AND SHALL NOT EXCEED 1/3 THE PLATE WIDTH.
5. MINIMUM NAILING SHALL COMPLY WITH TABLE 2304.9.1 OF THE INTERNATIONAL BUILDING CODE.
6. ALL PREFABRICATED METAL HANGERS AND CONNECTORS NOTED IN THE DRAWINGS SHALL BE "SIMPSON STRONG-TIE" CONNECTORS, WITH Z-MAX COATING, OR APPROVED EQUALS.
7. ALL WOOD STRUCTURAL MEMBERS SHALL BE TREATED AGAINST ROT AND INSECT DAMAGE. SEE SPECIFICATIONS.
8. PROVIDE 2x CONTINUOUS STRUCTURAL FASCIA AT ALL EAVES. FASCIA SUPPORTS HIPS AND LAST RAFTER ON GABLED ROOFS. DO NOT SPLICE FASCIA WITHIN 12" OF CORNERS.

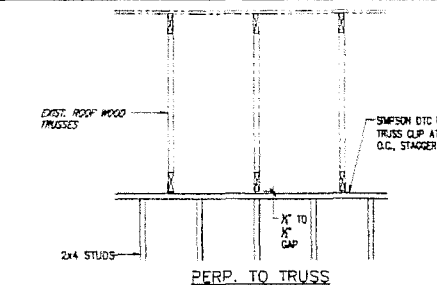
WOOD (CONTINUED):

9. PLYWOOD SHEATHING SHALL BE STRUCTURAL I CONFORMING TO U.S. COMMERCIAL STANDARD PS 1-74, EXCEPT AS NOTED OTHERWISE FOR SPECIAL SHEAR WALL AND DIAPHRAGM CONDITIONS. PROVIDE THE FOLLOWING MINIMUM ATTACHMENT AT ALL SUPPORTED EDGES.
1/2" PLYWOOD 8d AT 6" O.C.
3/4" PLYWOOD 10d AT 6" O.C.
5/8" PLYWOOD 10d AT 6" O.C.
 10. ALL PREFABRICATED METAL CONNECTORS SHALL BE BY SIMPSON STRONG-TIE COMPANY.
 11. ALL METAL CONNECTORS AND ANCHOR BOLTS SHALL BE HOT-DIPPED GALVANIZED.
- GLU-LAM BEAMS:**
1. ALL GLU LAMINATED BEAMS SHALL BE DOUGLAS FIR, 2,400 F SERIES, WITH EXTERIOR TYPE ADHESIVE.
 2. ALL BEAMS SHALL BE INDUSTRIAL GRADE, UNLESS NOTED OTHERWISE.
 3. ALL LAMINATIONS FOR "GLU-LAM" BEAMS SHALL BE 1-1/2" THICK AND OF WIDTH SHOWN OR NOTED. ALL LAMINATIONS SHALL BE PARALLEL TO THE BOTTOM EDGE OF THE BEAM.
 4. ALL GLU LAMINATED BEAMS SHALL DISPLAY AN A.L.T.C. QUALITY STAMP CERTIFYING COMPLIANCE WITH VOLUNTARY PRODUCT STANDARDS PS-55.
 5. ALL BEAMS SHALL HAVE STANDARD CAMBER BASED ON A RADIUS OF 1800 FEET UNLESS OTHERWISE NOTED ON PLAN.



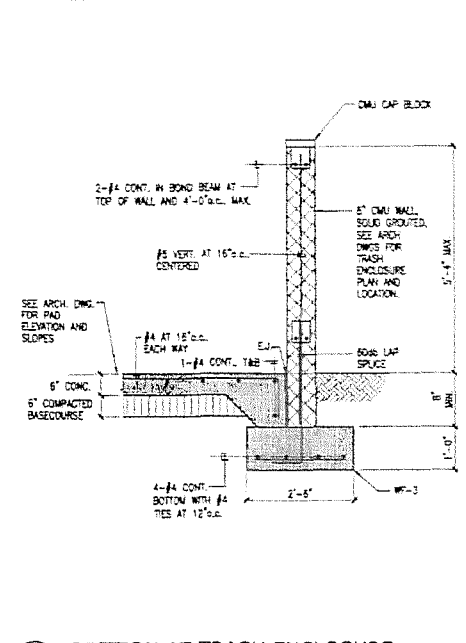
E TYPICAL HIP CORNER

50-02 NOT TO SCALE



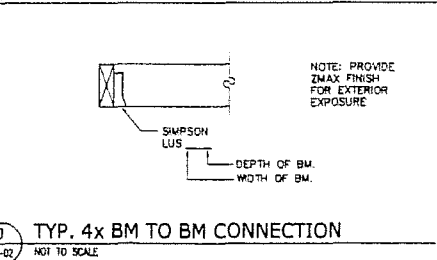
H TYP. NON-BEARING STUDWALL TO ROOF CONNECTION

50-02 NOT TO SCALE



F SECTION AT TRASH ENCLOSURE

50-02 SCALE: 3/4" = 1'-0"



J TYP. 4x BM TO BM CONNECTION

50-02 NOT TO SCALE

DATE	5/1/02	BY	SLH
PROJECT	UNIVERSITY OF HAWAII STATE OF HAWAII UNIVERSITY OF HAWAII MAUI COLLEGE 191 YOUNG STREET, 2ND FLOOR MAUI, HAWAII 96763		
PROJECT NUMBER	CC-11-5362	DATE	11-11-02
PROJECT NAME	MAUI HOSPITALITY ACADEMY RENOVATION	PROJECT	50-02
DESIGNED BY	SLH & ASSOC., INC.	CHECKED BY	SLH
DRAWN BY	SLH	IN CHARGE	SLH

THE WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION AND THE CORRECTION OF THIS PROJECT WILL BE UNDER MY OBSERVATION.
EXPIRATION DATE OF MY LICENSE: 12/31/16

SLH & ASSOC., INC.
191 YOUNG STREET, 2ND FLOOR
MAUI, HAWAII 96763

UNIVERSITY OF HAWAII
STATE OF HAWAII
UNIVERSITY OF HAWAII MAUI COLLEGE
191 YOUNG STREET, 2ND FLOOR
MAUI, HAWAII 96763

GENERAL NOTES

1. THE CONTRACTOR SHALL VERIFY ALL FIELD DIMENSIONS AND CONDITIONS PRIOR TO STARTING WORK. ALL DISCREPANCIES SHALL BE PROMPTLY REPORTED TO THE ENGINEER.
2. ALL OMISSIONS OR CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE WORKING DRAWINGS AND/OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ARCHITECT OR STRUCTURAL ENGINEER BEFORE PROCEEDING WITH ANY WORK SO INVOLVED.
3. ALL WORK SHALL CONFORM TO THE INTERNATIONAL BUILDING CODE (IBC), LATEST EDITION, AS ADOPTED BY THE STATE OF HAWAII.
4. UNLESS SPECIFICALLY DETAILED OTHERWISE, CONTRACTOR SHALL FOLLOW TYPICAL DETAILS ON SHEETS 50-01 THROUGH 50-03.
5. WHERE CONDITIONS VARY FROM THOSE SPECIFICALLY INDICATED BUT ARE SIMILAR TO DETAILS SHOWN ON SHEETS 50-01 THROUGH 50-03, MODIFY TYPICAL DETAILS TO MEET SPECIAL CONDITIONS.
6. STRUCTURAL DRAWINGS WHERE INDICATED HAVE BEEN DRAWN TO APPROXIMATE SCALE. UNLESS DRAWING SPECIFICALLY PROVIDES A DIMENSION, THE CONTRACTOR SHALL NOT USE OR SCALE STRUCTURAL DRAWINGS TO DETERMINE DIMENSIONS.
7. PRIOR TO ANY DEMOLITION AND DURING CONSTRUCTION, THE EXISTING STRUCTURES SHALL BE SHORED AS REQUIRED. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL SHORING. THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING FOR ALL STRUCTURAL MEMBERS AS REQUIRED FOR STRUCTURAL STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION.

REINFORCING STEEL

1. UNLESS OTHERWISE INDICATED ON PLANS AND/OR SCHEDULE, ALL REINFORCING STEEL SHALL BE HIGH STRENGTH GRADE DEFORMED BARS WHICH SHALL CONFORM TO THE STANDARD SPECIFICATION OF ASTM A630, GRADE 60. ALL REINFORCING STEEL TO BE WELDED SHALL CONFORM TO ASTM A706, GRADE 60. FOR SPECIAL CHORD REINFORCEMENT ASTM A515 GRADE 75 BARS SHALL BE USED AS SPECIFIED ON PLANS.
2. LOW HYDROGEN WELDING RODS SHALL BE USED FOR ALL WELDING TO REINFORCING BARS.
3. REINFORCING SHALL BE SPLICED ONLY AS SHOWN OR NOTED. SPLICES AT OTHER LOCATIONS SHALL BE MADE AWAY FROM POINTS OF MAXIMUM STRESS AND APPROVED BY THE ENGINEER. SEE DETAILS FOR "JAP SPLICES".
4. BARS NOTED AS "TYPICAL" VERTICAL AND HORIZONTAL WALL REINFORCING AND COLUMN REINFORCING SHALL HAVE A MINIMUM SPACING EQUAL TO 48 BAR DIAMETERS BUT NOT LESS THAN 24 INCHES.
5. WHERE MECHANICAL SPLICES ARE REQUIRED, SPLICES SHALL BE ABLE TO DEVELOP 125% OF THE YIELD STRENGTH OF THE BAR AND SHALL BE STAGGERED 3'-0" BETWEEN ADJACENT BARS.
6. PROVIDE CORNERS IN FOOTINGS THE SAME SIZE AND NUMBER AND IN THE SAME LOCATION AS VERTICAL COLUMN REINFORCING. CORNERS SHALL HAVE A MINIMUM PROTECTION EQUAL TO 48 BAR DIAMETERS BUT NOT LESS THAN 24 INCHES.
7. ALL REINFORCING STEEL, ANCHOR BOLTS AND OTHER INSERTS SHALL BE SECURED IN POSITION PRIOR TO PLACING CONCRETE.

MINIMUM CONCRETE COVER:
THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:

	COVER (IN)
(1) CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH	3"
(2) FORMED CONCRETE SURFACES EXPOSED TO EARTH OR WEATHER:	
NO. 6 THROUGH NO. 18 BARS	2"
NO. 5 BAR, W31 OR E31 WIRE, AND SMALLER	1 1/2"
(3) CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND, SLABS, WALL, JOISTS:	
NO. 14 AND NO. 18 BARS	1 1/2"
NO. 11 BAR AND SMALLER	3/4"
BEAMS, COLUMNS:	
PRIMARY REINFORCEMENT, TIES, STIRRUPS, SPIRALS	1 1/2"

C) CONCRETE COVER TOLERANCE SHALL BE AS FOLLOWS:

- (1) FORMED SOFFITS OR MEMBERS 4" OR LESS IN THICKNESS: $\pm 1/2"$ - $\pm 1"$
- (2) MEMBERS 12" THICK OR LESS: $\pm 3/8"$
- (3) MEMBERS GREATER THAN 12" THICK: $\pm 1/2"$

FOR ALL CASES, THE COVER SHALL NOT BE LESS THAN 1/2" OF WHAT IS INDICATED AND REQUIRED BY THE DRAWINGS AND SPECIFICATIONS.

D) WHEN THE IBC REQUIRES A THICKNESS OF COVER FOR FIRE PROTECTION GREATER THAN THE MINIMUM CONCRETE STATED ABOVE, SUCH GREATER THICKNESS SHALL BE USED.

CONCRETE BLOCK MASONRY

1. ALL MASONRY WORK SHALL CONFORM TO ACI 530-05.
2. CONCRETE BLOCK SHALL BE TYPE II HOLLOW LOAD BEARING UNITS.
3. MASONRY MORTAR SHALL BE TYPE "M" WITH A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2,500 PSI CONFORMING TO ASTM C270.
4. MASONRY GROUT SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2,500 PSI.
5. ALL CELLS SHALL BE GROUTED SOLID. GROUT IN LIFTS NOT TO EXCEED 8'-0". UNLESS NOTED OTHERWISE, ALL WALLS SHALL BE CONSTRUCTED IN CONVENTIONAL RUNNING BOND.
6. AT CMU WALLS PROVIDE HORIZONTAL JOINT REINFORCING AT 16" O.C.
7. WIRE POSTIONERS - CMU WALL REINFORCING SHALL BE SECURED AGAINST DISPLACEMENT BY GALVANIZED NO. 9 GAUGE WIRE POSTIONERS SPACED AT NOT MORE THAN 200 TIMES THE BAR DIAMETER.

CONCRETE

1. ALL CONCRETE WORK SHALL CONFORM TO ACI 318-05.
2. SLEEVES EXCEEDING ONE-THIRD THE SLAB OR WALL THICKNESS SHALL NOT BE PLACED IN STRUCTURAL CONCRETE UNLESS SPECIFICALLY DETAILED. PIPE MAY PASS THROUGH STRUCTURAL CONCRETE IN SLEEVES BUT SHALL NOT BE EMBEDDED THEREIN.
3. SECURE ALL BOLTS, ANCHORS, INSERTS, ETC. AND VERIFY ALL GROOVES, SLOTS, AND FINISHES PRIOR TO PLACING CONCRETE.
4. VERIFY LOCATIONS AND DIMENSIONS OF SLOTS, ANCHORS, DUCTS, ETC. RELATING TO MECHANICAL, ELECTRICAL AND ARCHITECTURAL WORK BEFORE POURING CONCRETE.
5. ALL CONCRETE WORK SHALL BE THOROUGHLY CONSOLIDATED DURING PLACEMENT USING A MECHANICAL VIBRATOR BACKUP (EXTRA) VIBRATOR(S) SHALL BE PRESENT AT EACH FOR IN CASE OF FAILURE.
6. ALL CONCRETE SHALL BE CURED FOR A PERIOD OF NOT LESS THAN 7 DAYS.
7. LOCATIONS OF ALL CONSTRUCTION OR COLD JOINTS SHALL BE COORDINATED WITH AND APPROVED BY THE ENGINEER.
8. 48 HOURS PRIOR TO THE POURING OF ANY STRUCTURAL CONCRETE, THE CONTRACTOR SHALL NOTIFY THE ENGINEER SO AN INSPECTION CAN BE MADE OF ALL FORMS AND REINFORCING STEEL.
9. THE 28-DAY COMPRESSIVE STRENGTH OF CONCRETE SHALL BE AS FOLLOWS:

	STRENGTH (PSI)	MAX. AGGREGATE	W/C RATIO
BUILDING SLAB ON GRADE, EXTERIOR WALKWAY AND LANDINGS	4,500	3/4"	0.45
FOOTINGS AND GRADE BEAMS	4,500	3/4"	0.45

EPOXYED ANCHORS:

1. ADHESIVE SHALL BE AN INDESTRUCTIBLE TWO-COMPONENT EPOXY SUCH AS SIMPSON SET-UP. HULT H-100 SD OR APPROVED EQUAL.
2. INSTALLATION OF EPOXYED ANCHORS SHALL FOLLOW ALL MANUFACTURER'S RECOMMENDATIONS.
3. THREADED ANCHOR RODS SHALL BE HOT DIP GALVANIZED AND CONFORM TO ASTM A-193, GRADE B. THREADED ANCHOR RODS SHALL BE LINE FULLY THREADED, CLEAN, STRAIGHT AND FREE OF INDENTATIONS OR OTHER DEFECTS ALONG THEIR LENGTH.
4. HOLES SHALL BE DRILLED WITH A CARBIDE-TIPPED DRILL BIT. IF HOLES ARE CORE DRILLED WITH DIAMOND-CORE BITS, EPOXY ADHESIVE SELECTED FOR USE BY THE CONTRACTOR SHALL BE SUITABLE FOR THIS APPLICATION WITH NO REDUCTION IN PUBLISHED ALLOWABLE SHEAR AND TENSION LOADS.
5. DIAMETER OF HOLES SHALL BE EQUAL TO THREADED ROD DIAMETER + 1/8".
6. ANCHORS SHALL BE INSTALLED IN CONCRETE/CMU THAT HAS REACHED ITS FULL DESIGN COMPRESSIVE STRENGTH.
7. INSTALLATION OF EPOXYED ANCHORS SHALL BE PERFORMED UNDER THE SUPERVISION OF A SPECIAL INSPECTOR IN ACCORDANCE WITH SECTIONS 1704.4 AND 1704.13 OF THE IBC. THE SPECIAL INSPECTOR MUST BE ON THE JOBSITE TO VERIFY ANCHOR TYPE, ANCHOR DIMENSIONS, CONCRETE TYPE, CONCRETE/CMU COMPRESSIVE STRENGTH, HOLE DIMENSIONS, HOLE CLEANING PROCEDURES, ANCHOR SPACING, EDGE DISTANCES, ANCHOR EMBEDMENT, AND TIGHTENING TORQUE.

DESIGN LOADS

1. SLOPED ROOF LIVE LOAD: 20 PSF
2. FLOOR LIVE LOADS:
RESIDENTIAL: 40 PSF
STAIRS: 100 PSF
3. WIND 105 MPH PER IBC, EXPOSURE C, $I_e = 1.0$, $K_{zt} = 1.6$
4. SEISMIC PER IBC, $S_a = 0.58$, $S_i = 0.252$, $I_e = 1.0$, SITE CLASS D
5. ALL COMPONENTS TO BUILDING SHALL BE SECURED PROPERLY TO RESIST LATERAL SEISMIC FORCES IN ACCORDANCE TO THE IBC. DESIGN OF ATTACHMENTS FOR COMPONENTS SHALL BE THE RESPONSIBILITY OF SUB-CONTRACTOR INSTALLING SUCH COMPONENT.

SPECIAL INSPECTION REQUIREMENTS LIST BELOW

1. SPECIAL INSPECTION REQUIREMENTS:
THE OWNER SHALL EMPLOY A SPECIAL INSPECTOR FOR WORK REQUIRING SPECIAL INSPECTION PER CHAPTER 17 OF THE 2006 IBC AS ADOPTED BY THE COUNTY OF MAUI.
2. SPECIAL INSPECTION FIRM SHALL HAVE AT LEAST 6 YEARS OF EXPERIENCE AND SHALL BE CERTIFIED WITHIN THE BUILDING DEPARTMENT JURISDICTION OF CONSTRUCTION (IF SUCH CERTIFICATION EXISTS).
3. THE CONTRACTOR SHALL COORDINATE HIS WORK WITH THE SPECIAL INSPECTOR TO ENSURE THAT ALL ELEMENTS REQUIRING SPECIAL INSPECTION ARE INSPECTED.
4. SPECIAL INSPECTION IS REQUIRED FOR THE FOLLOWING TYPES OF WORK:
A. CONCRETE:
OBSERVE THE TACKING OF TEST SPECIMENS AND THE PLACING OF CONCRETE FOR ALL ELEMENTS.
B. REINFORCING STEEL:
OBSERVE THE PLACEMENT OF ALL REINFORCING. THE SPECIAL INSPECTOR NEED NOT BE PRESENT CONTINUOUSLY DURING THE PLACING OF REINFORCING STEEL, PROVIDED HE HAS INSPECTED FOR CONFORMANCE WITH THE APPROVED PLANS PRIOR TO THE CLOSING OF FORMS FOR THE DELIVERY OF CONCRETE TO THE JOBSITE.
C. ANCHOR BOLTS FOR STEEL COLUMNS AND POST
D. EXPANSION BOLTS AND EPOXYED ANCHORS/REBAR
E. CONTINUOUS LOAD PATH AND SHEARWALL HOLD DOWNS

CONCRETE REPAIR/REHABILITATION

PROJECT CONDITIONS:

1. DO NOT APPLY MATERIAL IF IT IS RAINING OR IF RAIN IS IMMINENT.
2. PREPARATIONS SHOULD BE TAKEN TO AVOID DAMAGE TO ANY SURFACE NEAR THE WORK ZONE DUE TO SPILLAGE.
3. LEAVE FINISHED WORK IN NEAT, CLEAN CONDITION WITH NO EVIDENCE OF SPILL OVER ONTO ADJACENT SURFACES.

PRODUCTS:

1. ANTI-CORROSION PROTECTIVE COATING (CORROSION INHIBITOR): TWO-COMPONENT, ACRYLIC RESIN EPOXY SYSTEM FORMULATED TO INHIBIT THE CORROSION OF REINFORCING STEEL AND TO ACT AS A BONDING AGENT, SUCH AS: Sika ANACR 115 EPOXY, MASTER EMACO 100 OR APPROVED EQUAL.
2. FAST SETTING, HIGH STRENGTH PATCHING MORTAR (POLYMER MODIFIED MORTAR) FOR HORIZONTAL APPLICATIONS: ACRYLIC POLYMER-MODIFIED, PORTLAND CEMENT BASED, TRIM-L-GRADE PATCHING MORTAR, CONFORM TO REQUIREMENTS OF ASTM C 864, SUCH AS SIKATOP 122 PLUS, MASTER EMACO R310 OR T430, OR APPROVED EQUAL.
3. FAST SETTING, HIGH STRENGTH PATCHING MORTAR (POLYMER MODIFIED MORTAR) FOR VERTICAL APPLICATIONS: TWO-COMPONENT (OR SINGLE COMPONENT), ACRYLIC POLYMER-MODIFIED, PORTLAND CEMENT BASED, NON-SAG PATCHING MORTAR, CONFORM TO REQUIREMENTS OF ASTM C 864, SUCH AS SIKATOP 123 PLUS, MASTER EMACO H425, OR APPROVED EQUAL.

PREPARATION - GENERAL:

1. AD 503R, AD 503L, AD 503L FOR EPOXY RESIN SYSTEMS. MIX THE PATCHING MATERIALS WITH OR WITHOUT FILLERS IN ACCORDANCE WITH THE MANUFACTURER'S INSTRUCTIONS. ALL APPLICATIONS OF THE MIXED MATERIALS SHALL BE PERFORMED WITHIN THE WORKING LIFE OF THE PATCHING SYSTEM. UNUSED MIXED MATERIALS WHICH HAVE REACHED THE END OF THE WORKING OR POT LIFE SHALL BE REMOVED FROM THE JOB SITE AT THE CONTRACTOR'S EXPENSE. FIELD MIXING AND SIZE OF BATCH SHALL BE DETERMINED BY THE CONTRACTOR. PATCHING SYSTEMS SHALL BE PROVIDED AS INDICATED AND REQUIRED BY SPECIFICATIONS.
2. SURFACES PREPARE ALL SURFACES REQUIRING PATCHING SO THAT THEY WILL BE STRUCTURALLY SOUND, CLEAN, AND FREE OF DIRT, LOOSE CONCRETE, LOOSE MORTAR PARTICLES, PAINT, FLUX, PROTECTIVE COATINGS, EFFLORESCENCE, LANTANE, AND OTHER MATTER DETRIMENTAL TO PROPER ADHESION.
3. CHECK AREAS AROUND CRACKS, BUSTERS, OLD PATCHES, AND RUST SPOTS BY TAPPING SURFACE WITH A HAMMER. REMOVE ALL BLUNT INSTRUMENTS, OR USING A CHAIN, SOUND FOR LOOSE OR HOLLOW AREAS AND MARK AREAS FOR REMOVAL OF UNSOUND CONCRETE OR MORTAR.
4. ALL AREAS WHERE CRACKS EXIST AND THE CONCRETE IS NOT IN A STATE OF CONCRETE DELAMINATION AS DETERMINED BY SOUNDING WITH A HAMMER OR CHAIN SHALL BE CLEANED OF ALL DIRT, LOOSE MORTAR PARTICLES, PAINT, FLUX, LANTANE, AND ALL OTHER MATTER DETRIMENTAL TO PROPER ADHESION.
5. REMOVE UNSOUND MATERIALS AND FILMS BY MEANS OF CHIPPING, SAW CUTTING, BUSH HAMMERING, NEEDLE GUNNING, SAND BLASTING, OR OTHER SUITABLE METHOD.
6. WASH SURFACES FROM WORK SURFACES WITH CLEAN WATER REMOVE ANY REMAINING EFFLORESCENCE BY ADD WASH, MURATIC OR SIMILAR, AND RINSE WITH CLEAN WATER.

PREPARATION - REINFORCEMENT:

1. MECHANICALLY REMOVE ALL RUST AND LOOSE MATERIAL FROM ALL EXPOSED RUSTED REINFORCING STEEL TO SOUND METAL BY POWER BRUSHING, NEEDLE SCALER, OR SAND BLASTING.
2. APPLY ANTI-CORROSION PROTECTIVE COATING ACCORDING TO MANUFACTURER'S PRINTED INSTRUCTIONS TO ALL EXPOSED AND CLEANED REINFORCEMENT BARS AND CONCRETE SURFACES TO RECEIVE PATCH.
3. USING BRUSH OR ROLLER, SCRUB PRIMER BONDING AGENT ONTO WORK SURFACES WHILE ALSO COMPLETELY COATING EXPOSED REINFORCING STEEL AND CONCRETE.
4. DO NOT RESURFACE OVER HARDENED PRIMER BONDING AGENT. IF PRIMER BONDING AGENT BECOMES TACK FREE, MECHANICALLY ROUGHEN SURFACE AND REPRIME.

SPALL REPAIR (INCLUDES CRACKED/CLOSED SPALL CORNER OF PIERCEMENT):

1. SAW CUT OR ROUT OUT SPALLS TO A MINIMUM WIDTH AND DEPTH OF 1/2 INCH.
2. REFER TO GENERAL PREPARATION INSTRUCTIONS ABOVE.
3. IF REINFORCING BARS ARE EXPOSED, REFER TO REINFORCING PREPARATION INSTRUCTIONS ABOVE.
4. MECHANICALLY SCOUR WORK SURFACES TO OBTAIN APPROPRIATE FRACTURE WITHIN A MINIMUM SURFACE PROFILE OF PLUS OR MINUS 1/8 INCH.
5. APPLY PRIMER BONDING AGENT PER ABOVE INSTRUCTIONS AND MANUFACTURER'S PRINTED INSTRUCTIONS.
6. WHILE PRIMER BONDING AGENT IS STILL TACKY, MIX AND APPLY PREPARED PATCHING MORTAR TO PREPARED SPALL FOLLOWING MANUFACTURER'S PRINTED INSTRUCTIONS.
7. PLACE PATCHING MATERIAL IN THIN LAYERS AS RECOMMENDED BY THE MANUFACTURER. EACH INTERMEDIATE LAYER SHALL BE CROSS-SCOURED FOR MECHANICAL BONDING. ALL LAYERS FOR EACH PATCH SHALL BE PLACED ON THE SAME DAY. USE HAND TAMPERS TO CONSOLIDATE THE PATCHING MATERIAL. LEVEL EACH LAYER AND SPEED THE FINAL SURFACE REMOVE EXCESS PATCHING MATERIAL ON ADJACENT SURFACES BEFORE IT HARDENS. DO NOT FEATHER OUT ONTO ADJACENT SURFACES. UPON COMPLETION OF FINISHING OPERATIONS, APPLY EVAPORATION REDUCER AND THEN APPLY CURING COMPOUND IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. PROTECT PATCHES WITH WATERTIGHT FORMS AND/OR COVERS UNTIL THE PATCHES ARE PROPERLY CURED.
8. USE PROPER PATCHING MATERIAL DEPENDING ON IF THE LOCATION OF THE SPALL IS HORIZONTAL, VERTICAL OR OVERHEAD.
9. MINIMUM DEPTH OF SPALL PATCH OVER REINFORCING SHALL BE 1 INCH.
10. DO NOT FEATHER EDGES OF SPALL PATCH. SAW CUT OR ROUT EDGE OF REPAIR TO OBTAIN A 1/2 INCH MINIMUM VERTICAL EDGE TO REPAIRED AREA.
11. FINISHED PATCHED SPALL SHOULD MATCH EXISTING SURFACES IN TEXTURE AND APPEARANCE.
12. AT EXPOSED AREAS, GRIND THE REPAIRED PATCH FLUSH WITH SURROUNDING SURFACES. PATCH TEXTURE TO MATCH EXISTING.

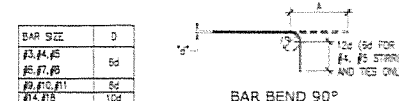
ABBREVIATIONS

A.B.	ANCHOR BOLT	HSS	HIGH STRENGTH BOLT																				
ARCH.	ARCHITECTURAL OR ARCHITECT	INT.	INTERIOR																				
BAL.	AT BALANCE	JOINT	JOINT																				
BOT.	BOTTOM	KSF	KIPS PER SQUARE FOOT																				
BSU	BEAM	LONG.	LONGITUDINAL																				
B.T.B.	BASALTIC THERMITE BARRIER	MAX.	MAXIMUM																				
C.I.P.	CAST IN PLACE	M.E.	MECHANICAL																				
C.J.	CONSTRUCTION OR CONTROL JOINT	MIN.	MINIMUM																				
C.J.P.	COMPLETE JOINT PENETRATION WELD	N.T.S.	NOT TO SCALE																				
CL	CENTER LINE	NO.	NUMBER																				
CLR	CLEAR	O.C.	ON CENTER																				
CMU	CONCRETE MASONRY UNIT	OPP.	OPPOSITE																				
CONC.	CONCRETE	PL.	PLATE																				
CONT.	CONTINUOUS	PSF	POUNDS PER SQUARE INCH																				
CORN.	CORNER	PSF	POUNDS PER SQUARE FOOT																				
DB	REBAR DIAMETER	R.F. PR	REINFORCEMENT																				
DIA. OR Ø	DIAMETER	REQ'D	REQUIRED																				
DIMS.	DIAGONAL DIMENSIONS	SHT.	SHEET																				
DET.	DETAIL	SP	SPACE OR SPACES																				
EA.	EACH	SM.	SIMILAR																				
E.L.	ELEVATION	SO.	SQUARE																				
E.F.	ELEVATOR	STD.	STANDARD																				
ELEV.	ELEVATION	T & B	TOP AND BOTTOM																				
EQ.	EQUAL	THK.	THICK																				
EXT.	EXTERIOR	T.O.C.	TOP OF CONCRETE																				
EXT.	EXTERIOR	T.O.F.	TOP OF FOOTING																				
F.P.	FOOT OF FEET	T.O.S.	TOP OF SLAB																				
FT.	FOOTING	T.O.W.	TOP OF WALL																				
GA.	GAGE	TRANS.	TRANSVERSE </tr <tr> <td>GALV.</td><td>GALVANIZED</td><td>TYPE</td><td>TYPICAL</td></tr> <tr> <td>H.D.G.</td><td>HOT DIPPED GALVANIZED</td><td>VERT.</td><td>VERTICAL</td></tr> <tr> <td>HORIZ.</td><td>HORIZONTAL</td><td>W/O</td><td>WITH OUT</td></tr> <tr> <td></td><td></td><td>W.I.</td><td>WELDED WIRE FABRIC</td></tr> <tr> <td></td><td></td><td>W.W.M.</td><td>WELDED WIRE MESH</td></tr>	GALV.	GALVANIZED	TYPE	TYPICAL	H.D.G.	HOT DIPPED GALVANIZED	VERT.	VERTICAL	HORIZ.	HORIZONTAL	W/O	WITH OUT			W.I.	WELDED WIRE FABRIC			W.W.M.	WELDED WIRE MESH
GALV.	GALVANIZED	TYPE	TYPICAL																				
H.D.G.	HOT DIPPED GALVANIZED	VERT.	VERTICAL																				
HORIZ.	HORIZONTAL	W/O	WITH OUT																				
		W.I.	WELDED WIRE FABRIC																				
		W.W.M.	WELDED WIRE MESH																				



BAR BEND 135°

STANDARD HOOK 180°



BAR BEND 90°

BAR SIZE	MINIMUM EXTENSION LENGTHS "A"			
	STANDARD HOOKS	90° HOOKS	180° HOOKS	135° HOOKS
#3	180"	40"	40"	40"
#4	5'	4'	4'	4'
#5	5'	11'	7'	5'
#6	11'	13'	14'	12'
#7	12'	15'	16'	14'
#8	14'	17'	19'	16'

A TYPICAL REBAR BENDING

50-00 NOT TO SCALE

REGION	DATE	DESCRIPTION	BY	CHECK

This work was prepared by me or under my supervision and completion of this project will be under my observation. Expiration Date of the License: 12/31/16

Emily S. Pichler

UNIVERSITY OF HAWAII
STATE OF HAWAII

UNIVERSITY OF HAWAII MAUI COLLEGE
ON PROJECT NUMBER CC-111-5382
2248A & 2248B HOSPITALITY ACADEMY RENOVATION
KAILUA, MAUI

GENERAL NOTES AND TYPICAL DETAILS

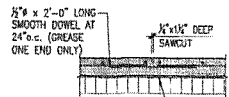
ELSA B. ASSOC., INC.
1895 YOUNG STREET, 2ND FLOOR
HONOLULU, HAWAII 96813

PROJECT NO. CC-111-5382
DATE: 05-11-2012

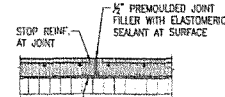
ISSUED BY: [Signature]
CHECKED BY: [Signature]
DATE: 05-20-2012

S0-00

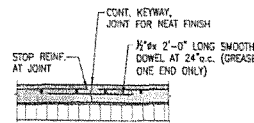
BY: 48 x 102 sheets



TYP. CONTROL JOINT (C.J.)



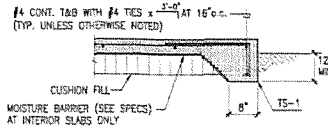
TYP. EXPANSION JOINT (E.J.)



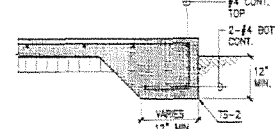
TYP. CONSTRUCTION JOINT

SAWCUT SHALL BE PERFORMED WITHIN 8 HOURS OF POUR AND AS SOON AS SLAB HAS SET AND IS FIRM ENOUGH SUCH THAT SAWCUT BLADE DOES NOT RIP, TEAR, OR DAMAGE SLAB OR UNRAVEL EDGES

A TYPICAL SLAB JOINT DETAILS
50-01 NOT TO SCALE



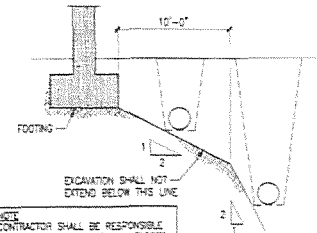
TS-1



TS-2

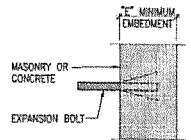
1. THIS DETAIL IS INTENDED TO SHOW DIMENSION OF THICKENED EDGE SLAB ONLY.
2. ALL SIDEWALK EDGES SHALL HAVE TS-1.

B THICKENED SLAB DETAIL (TS-1 and TS-2)
50-01 NOT TO SCALE



NOTE: CONTRACTOR SHALL BE RESPONSIBLE FOR MAINTAINING ALL TRENCH SLOPES BY SHORING AS REQUIRED.

C TRENCH DETAIL
50-01 NOT TO SCALE

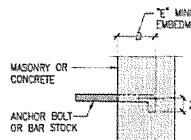


NOTES:

1. GROUT ALL CELLS WITH BOLTS FULL HEIGHT.
2. ALL EXPANSION BOLTS SHALL BE HILTI KWIK BOLT II, HOT DIP GALV. ANCHORS OR EQUAL, HAVING THE FOLLOWING MINIMUM LOAD VALUES.
3. I.T.C. REPORT SHALL BE SUBMITTED FOR REVIEW.

BOLT SIZE	MINIMUM "E"	MINIMUM ALLOW. SHEAR IN CONC. (K)	MINIMUM ALLOW. SHEAR IN MASONRY (K)
1/2" DIA.	4"	1550	550
3/8" DIA.	4"	1800	650
1/2" DIA.	5"	2100	800
3/4" DIA.	6"	2500	1000
1" DIA.	7"		

D EXPANSION BOLT
50-01 NOT TO SCALE

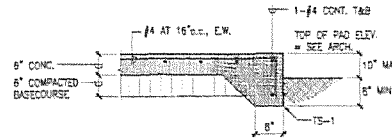


NOTES:

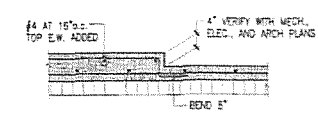
1. GROUT ALL CELLS WITH BOLTS FULL HEIGHT.
2. UNLESS NOTED OTHERWISE, THESE ANCHOR BOLT DETAILS SHALL APPLY.
3. ALL ANCHOR BOLTS SHALL BE HOT DIPPED GALVANIZED.

BOLT SIZE	MINIMUM "E"
1/2" DIA.	6"
3/8" DIA.	6"
1/2" DIA.	12"
3/4" DIA.	12"

E ANCHOR BOLT
50-01 NOT TO SCALE

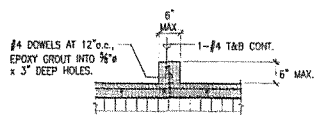


AT EXTERIOR CONDITION



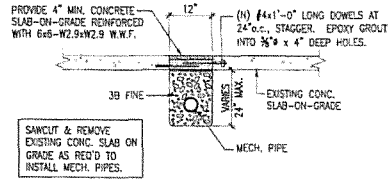
INTERIOR SLAB-ON-GRADE CONDITION

F TYPICAL RAISED PAD AT SLAB-ON-GRADE CONDITION
50-01 NOT TO SCALE



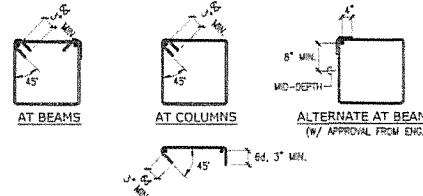
PLAN

G TYP. CURB AT BASE OF STUD WALL
50-01 NOT TO SCALE

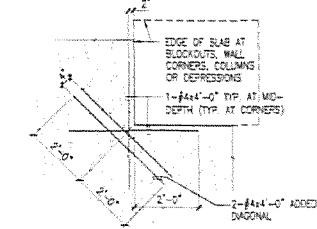


REPAIR OF CONC. SLAB-ON-GRADE FOR MECH. PIPE INSTALLATION

H SLAB ON GRADE REPAIR DETAIL
50-01 NOT TO SCALE

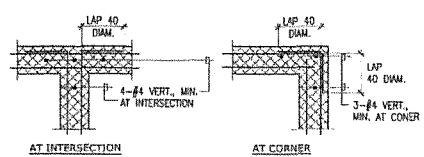


J TYPICAL STIRRUP AND TIE DETAIL
50-01 NOT TO SCALE



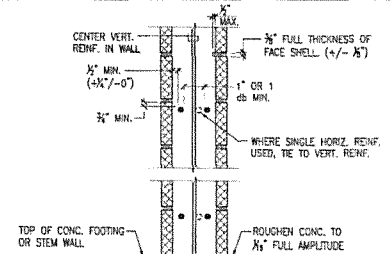
PLAN

K TYP. S.O.G. RE-ENTRANT CORNER/BLOCKOUT DETAIL
50-01 NOT TO SCALE



TWO LAYERS OF REINF.

L TYP. CMU WALL BOND BEAM REINFORCING
50-01 NOT TO SCALE

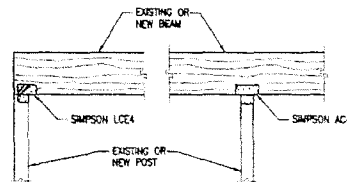
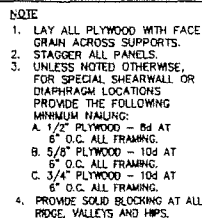


M TYPICAL CMU DETAIL
50-01 NOT TO SCALE

BAR SIZE	f'c = 3000 PSI		f'c = 4000 PSI	
	TOP BARS	OTHER BARS	TOP BARS	OTHER BARS
#3	24"	24"	24"	24"
#4	24"	24"	24"	24"
#5	30"	30"	31"	24"
#6	43"	33"	37"	29"
#7	63"	48"	54"	42"
#8	72"	55"	62"	49"
#9	81"	62"	70"	54"

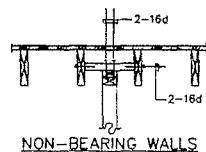
N TYPICAL LAP SPlice DETAIL
50-01 NOT TO SCALE

UNIVERSITY OF HAWAII STATE OF HAWAII		PROJECT NO.		DATE	
UNIVERSITY OF HAWAII COLLEGE OF HONOLULU 2245A & 2245B HOSPITALITY ACADEMY RENOVATION KAAHULU, HAWAII		PROJECT NO. CO-11-5362		DATE 1-30-2013	
TYPICAL DETAILS		SLSH & ASSOC., INC. 1914 YOUNG STREET, 2ND FLOOR HONOLULU, HAWAII		PROJECT NO. CO-11-5362	
This work was prepared by me or under my supervision and representation of this project is by under my representation. Expiration Date of License: 12/31/12		DESIGNED BY SLSH		CHECKED BY SLSH	
DRAWN BY SLSH		APPROVED BY SLSH		DATE 1-30-2013	
SHEET NO. 12 OF 123 SHEETS		S0-01			

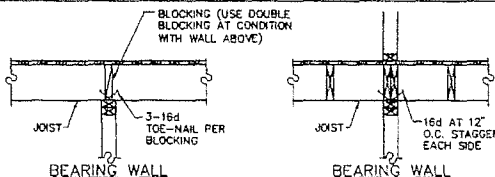


SD-02 NOT TO SCALE

SD-02 NOT TO SCALE



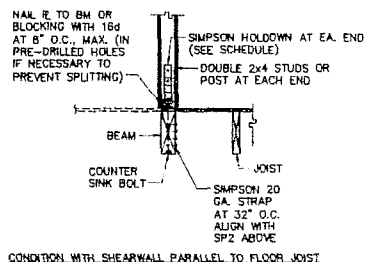
NON-BEARING WALLS



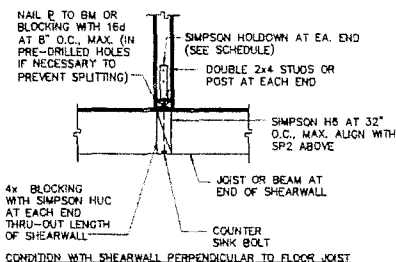
BEARING WALL

BEARING WALL

90-07	NOT TO SCALE
-------	--------------



CONDITION WITH SHEARWALL PARALLEL TO FLOOR JOIST



CONDITION WITH SHEARWALL PERPENDICULAR TO FLOOR JOIST

FOR SHEARWALLS ON 2nd FLR WHICH DOES NOT CONTINUE TO 1st FLOOR

80-82 NOT TO SCALE

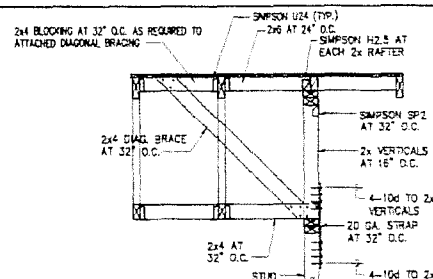
WORD:

1. ALL WOOD STRUCTURAL MEMBERS SHALL BE DOUGLAS FIR AND SHALL MEET THE FOLLOWING MINIMUM GRADES AS ESTABLISHED BY THE WEST COAST LUMBER INSPECTION BUREAU:

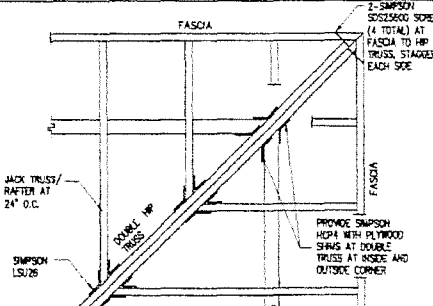
BEAMS	NO. 1
JOISTS/PARTIES/TRUSS	NO. 1
POSTS AND STUDS	NO. 1
BLOCKING, PLATES, LEDGERS ETC.	NO. 2
2. PROVIDE 30# FET BELOW ALL PLATES RESTING ON CONCRETE OR MASONRY.
3. PROVIDE STANDARD WASHERS AT ALL BOLTS AND NUTS BEARING ON WOOD.
4. HOLES THRU PLATES AND STUDS SHALL BE CENTERED IN THE MEMBER AND SHALL NOT EXCEED 1/4" DIA.
5. MINIMUM WINDING SHALL COMPLY WITH TABLE 2304.9.1 OF THE INTERNATIONAL BUILDING CODE
6. ALL PREPARED METAL HANGERS AND CONNECTORS NOTED IN THE DRAWINGS SHALL BE "SIMPSON STRONG-TIE" CONNECTORS, WITH 2-MAX COATING, OR APPROVED EQUIT.
7. ALL WOOD STRUCTURAL MEMBERS SHALL BE TREATED AGAINST ROT AND INSECT DAMAGE. SEE DETAIL.
8. PROVIDE 2X CONTINUOUS STRUCTURAL FASCIA AT ALL EAVES. FASCIA SUPPORTS HIPS AND LAST NAILER ON GABLE ROOFS. DO NOT SPURCE FASCIA WITHIN 12" OF CORNERS.

WOOD (CONTINUED):

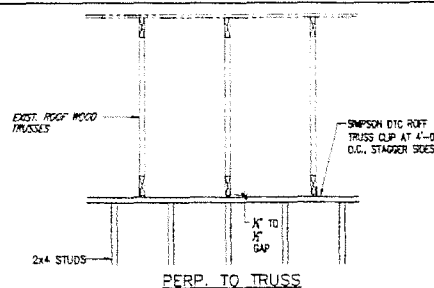
9. PLYWOOD SHEATHING SHALL BE STRUCTURAL I CONFORMING TO U.S. COMMERCIAL STANDARD PS-1-74. EXCEPT AS NOTED OTHERWISE FOR SPECIAL SHEAR WALL AND DIAPHRAGM CONDITIONS, PROVIDE THE FOLLOWING MINIMUM ATTACHMENT AT ALL SUPPORTED EDGES.
- | | |
|------------------------|----------------|
| 1" PLYWOOD | 86 at 6" o.c. |
| 3/4" PLYWOOD | 104 at 6" o.c. |
| 1/2" PLYWOOD | 104 at 6" o.c. |
10. ALL PREFABRICATED METAL CONNECTIONS SHALL BE BY SIMPSON STRONG-TIE COMPANY.
11. ALL METAL CONNECTORS AND ANCHOR BOLTS SHALL BE HOT-DIPPED GALVANIZED.
- GLU-LAM BEAMS:**
1. ALL GLUE LAMINATED BEAMS SHALL BE DOUGLAS FIR 2,400 F SERIES, WITH EXTERIOR TYPE ADHESIVE.
 2. ALL BEAMS SHALL BE INDUSTRIAL GRADE, UNLESS NOTED OTHERWISE.
 3. ALL LAMINATIONS FOR "GLU-LAM" BEAMS SHALL BE 1-1/2" THICK AND OF WEST SHOWN OR NOTED.
 4. ALL LAMINATIONS SHALL BE PARALLEL TO THE BOTTOM EDGE OF THE BEAM.
 5. ALL GLUE LAMINATED BEAMS SHALL DISPLAY AN A.S.T.C. QUALITY STAMP CERTIFYING COMPLIANCE WITH VOLUNTARY PRODUCT STANDARDS PS-S6.
 6. ALL BEAMS SHALL HAVE STANDARD CAMBER BASED ON A RADIUS OF 1800 FEET UNLESS OTHERWISE



SG-02 NOT TO SCALE

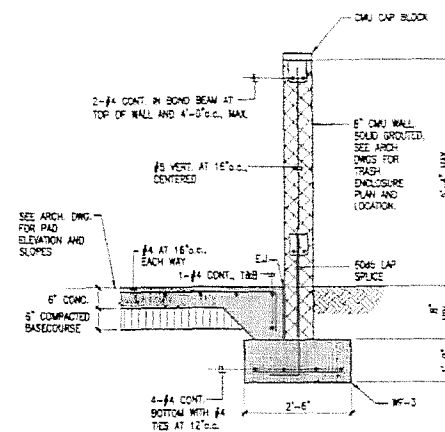


E	Y	F	I	C	A

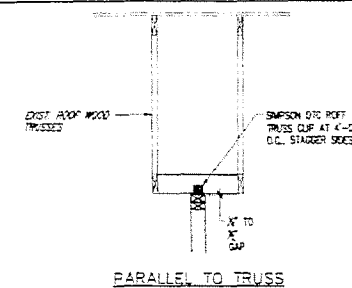


PERP. TO TRUSS

III. IV.

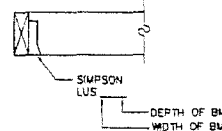


50-02 SCALE: 3/4" = 1'-0"



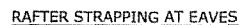
PARALLEL TO TRUSS

SP-03	NOT TO SCALE
-------	--------------

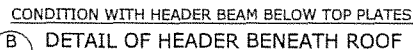


NOTE: PROVIDE
ZMAX FINISH
FOR EXTERIOR
EXPOSURE

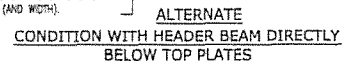
UNIVERSITY NO.	DATE	DESCRIPTION	SHEET OF	CITY
 UNIVERSITY OF HAWAII SCHOOL OF BUSINESS				
				
UNIVERSITY OF HAWAII MAUI COLLEGE 144 PROJECT NUMBER 00-11-5362 224A & 224B HOSPITALITY ACADEMY RENOVATION KAHAKU, MAUI				
TYPICAL DETAILS				
This work was prepared for the use of and/or the supervision and construction of this project will be under my direct supervision. Expiration date of the license 4/30/06. <i>Handwritten Signature: Douglas J. Smith</i>				
SLISH & ASSOC., INC. 1404 N. MOLOKAI RD. 2ND FLOOR MOLOKAI, HAWAII 96768		PROJECT NO. 00-11-5362		DESIGNED BY S-01
DRAWN BY BOK		APPROVED BY JL		DATE 10-20-05



A
SQ-03



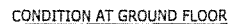
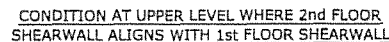
SO-03 SCALE: 3/4" = 1'-0"



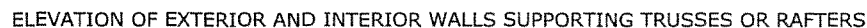
TYP. SECTION AT EXTERIOR WALL



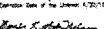
SCALE: $3/4" = 1'-0"$

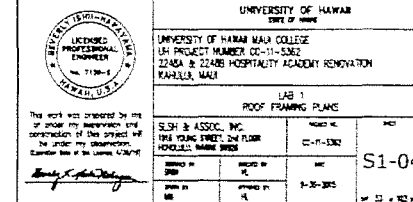
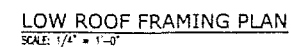
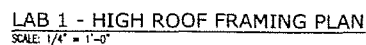


50-03 NOT TO SCALE

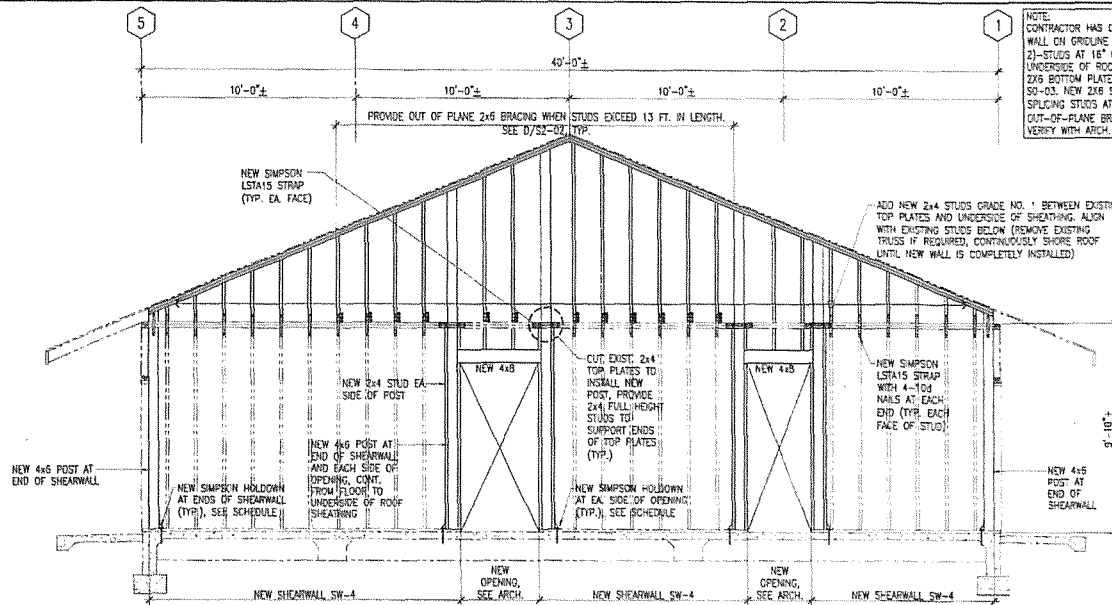


NOT TO SCALE

PROJECT NO.	TITLE	DESCRIPTION	SHEET NO.	TOTAL SHEETS
		UNIVERSITY OF HAWAII STATE OF HAWAII UNIVERSITY OF HAWAII MAUI COLLEGE 124 PROJECT NUMBER 02-11-5362 2248A & 2248B HOSPITALITY ACADEMY RENOVATION MAUI, HAWAII		
THIS WORK WAS PREPARED BY ME OR UNDER MY SUPERVISION AND CONSTRUCTION OF THIS PROJECT IS TO BE UNDER MY SUPERVISION. EXPIRATION DATE OF LICENSE 4/30/15		TYPICAL DETAILS SEBY & ASSOC., INC. 121 HIGGINS STREET AND FLOOR MAUI, HAWAII 96761 PREPARED BY 02-11-5362 DATE 1-30-2015		
		SO-01		

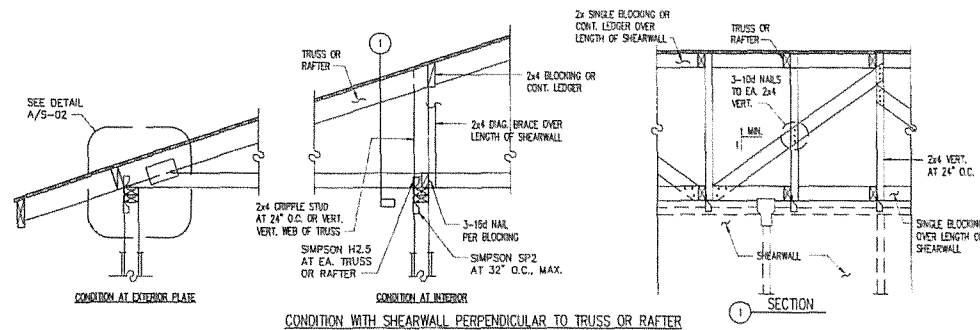
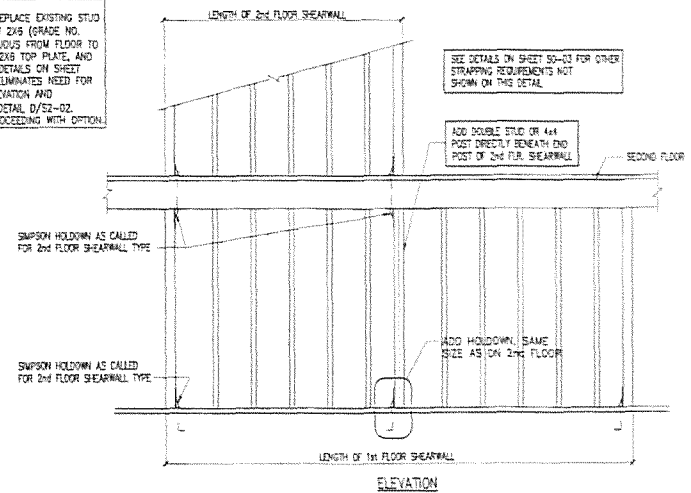


1/4" x 1" - 0"

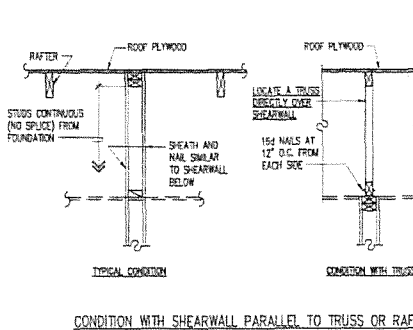


A WALL ELEVATION ON GRIDLINE B AT REGISTRATION LOBBY
 S2-02 SCALE: 3/8" = 1'-0"

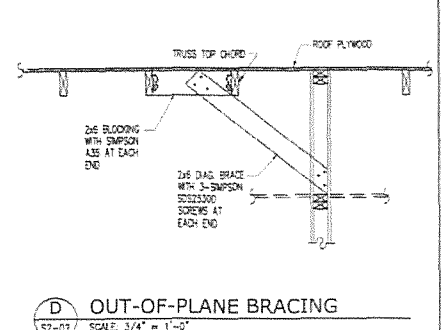
B CONDITION WHEN END(S) OF 2ND FLOOR AND 1ST FLOOR SHEARWALL DO NOT ALIGN
 S2-02 NOT TO SCALE



C TYPICAL SHEARWALL CONNECTIONS TO ROOF
 S2-02 SCALE: 3/4" = 1'-0"

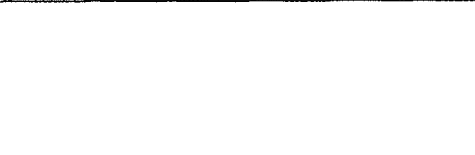
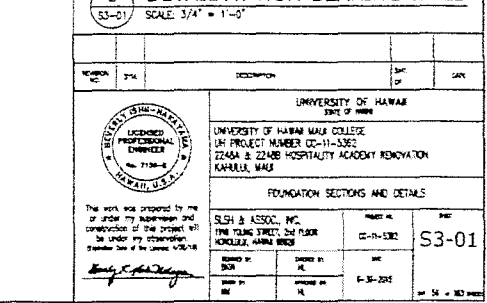
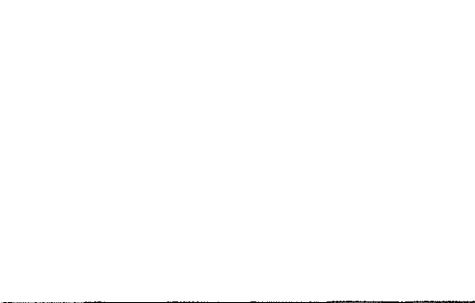
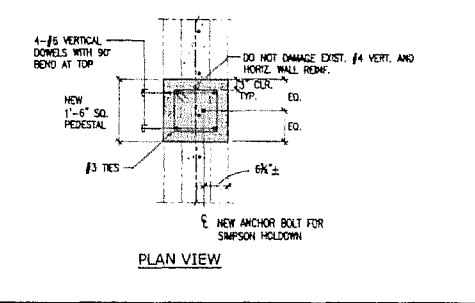
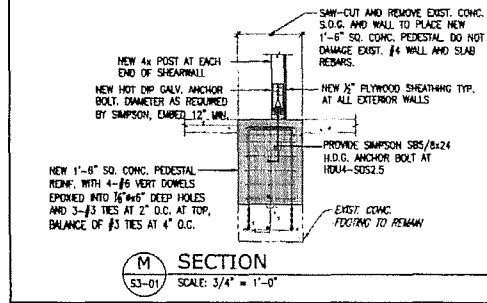
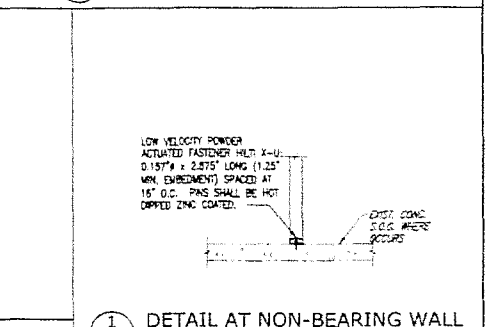
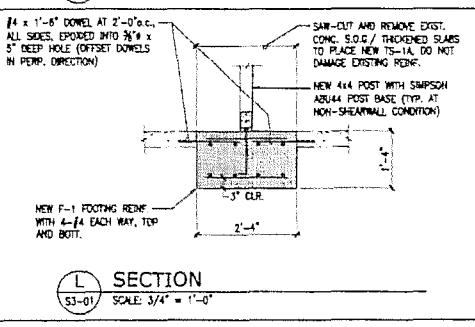
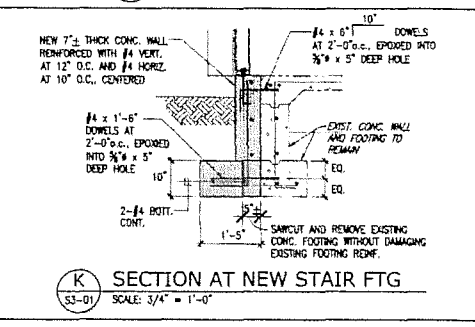
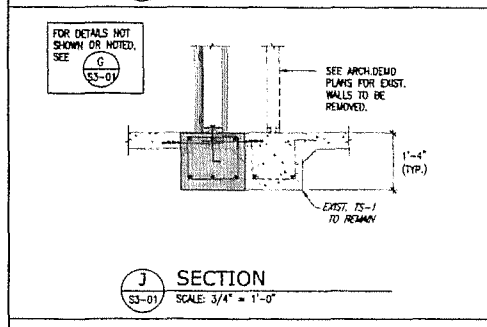
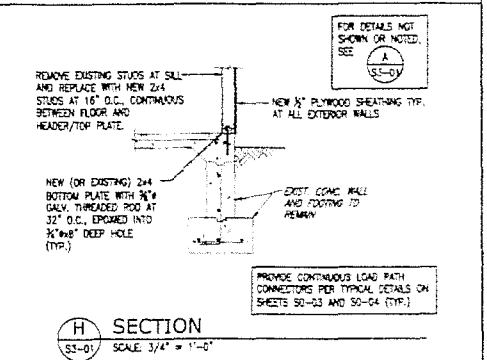
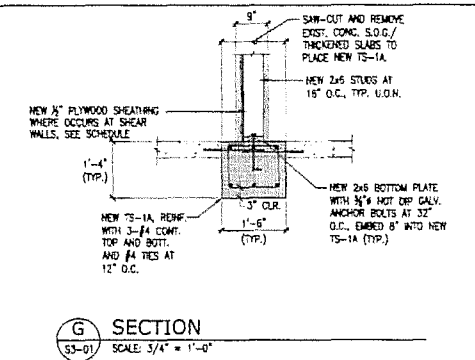
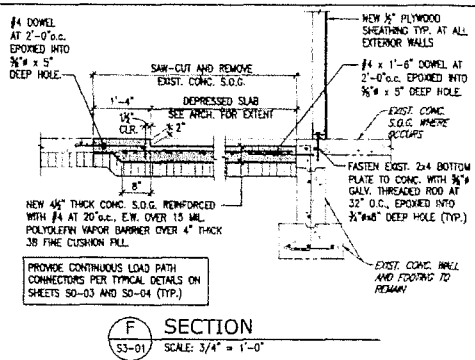
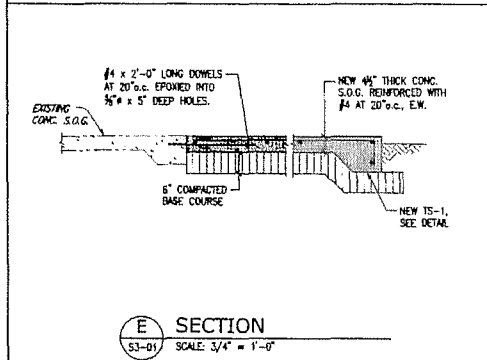
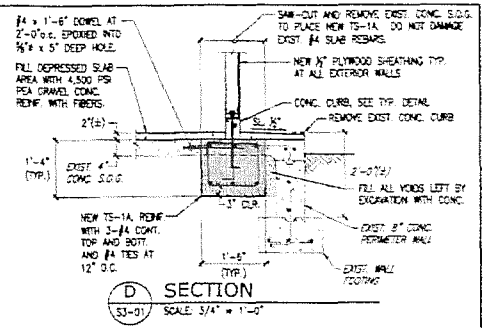
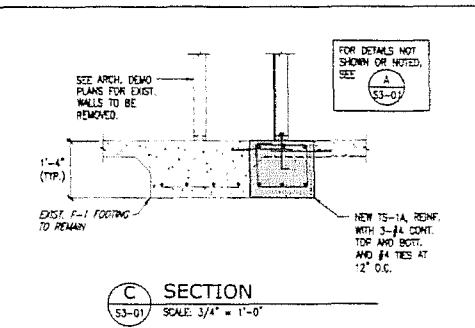
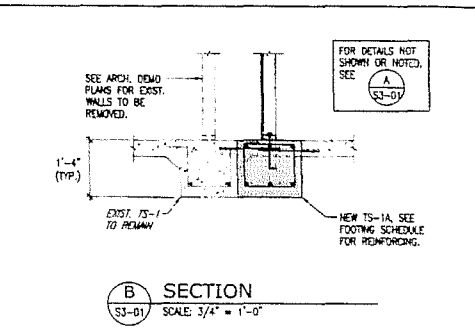
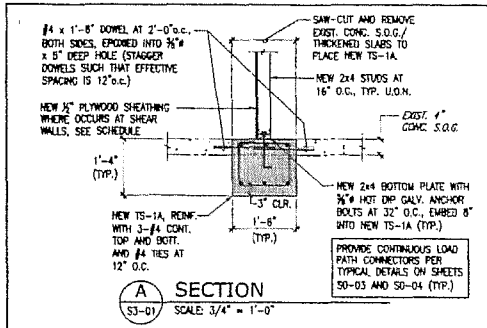


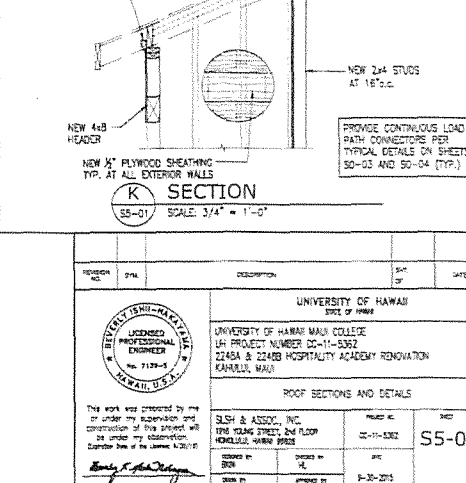
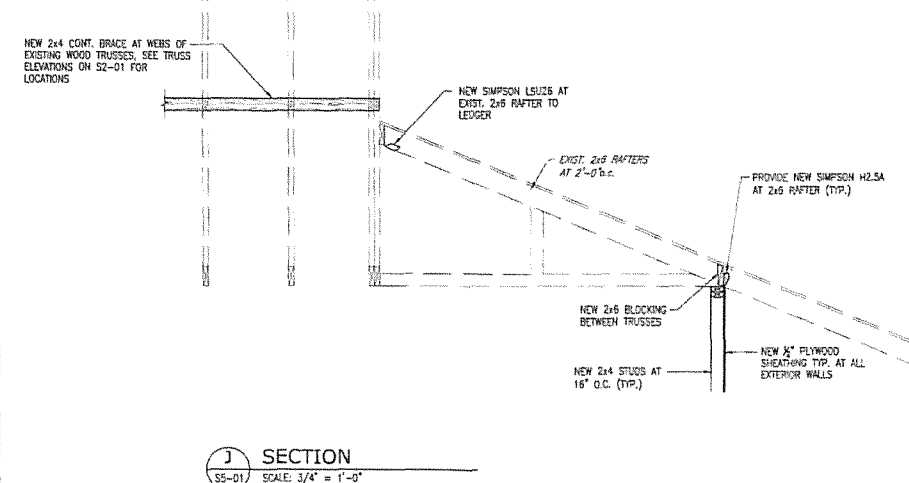
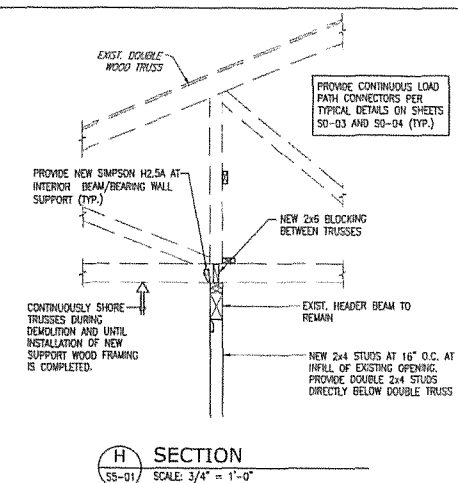
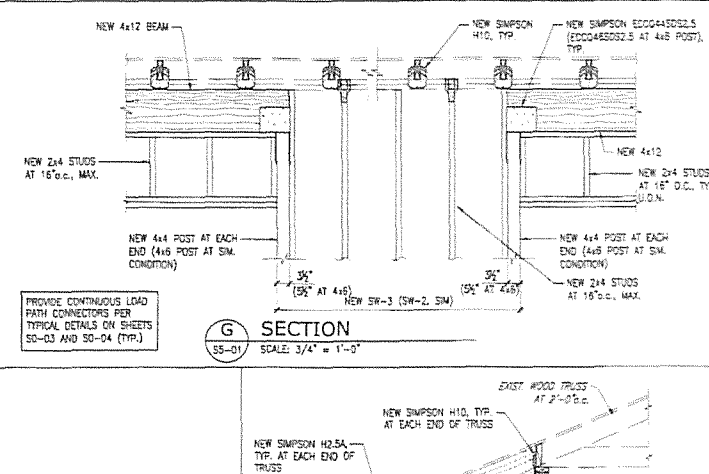
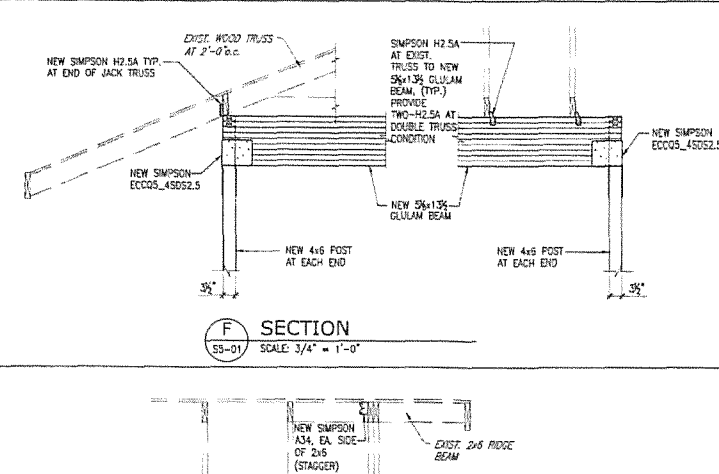
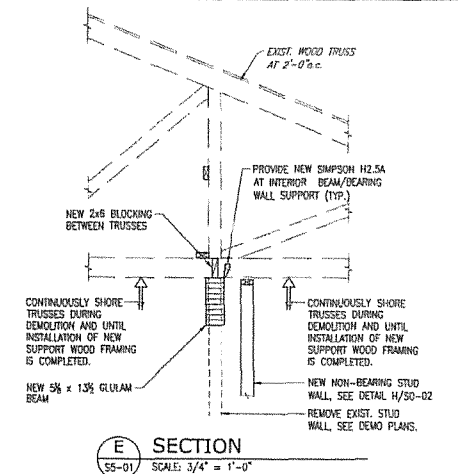
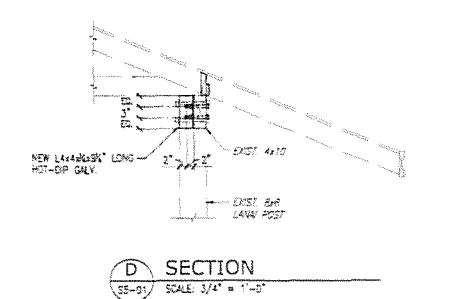
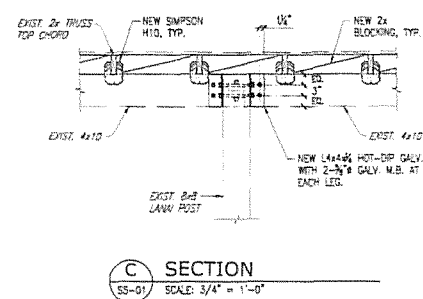
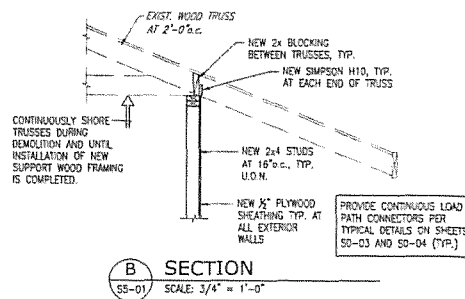
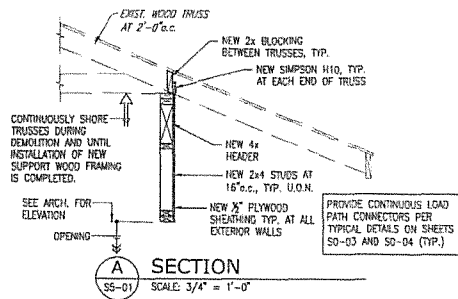
CONDITION WITH SHEARWALL PARALLEL TO TRUSS OR RAFTER



D OUT-OF-PLANE BRACING
 S2-02 SCALE: 3/4" = 1'-0"

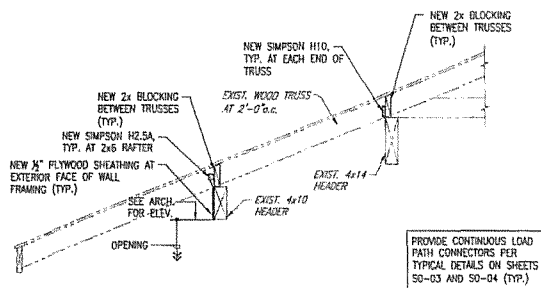
REVISION	DATE	DESCRIPTION	BY	CHKD
UNIVERSITY OF HAWAII STATE OF HAWAII				
UNIVERSITY OF HAWAII MANA COLLEGE 1750 YOUNG STREET, 2ND FLOOR KOHALA, HAWAII 96751				
WALL ELEVATION AT REGISTRATION LOBBY				
SLSH & ASSOC., INC. 1750 YOUNG STREET, 2ND FLOOR KOHALA, HAWAII 96751		PROJECT NO.	CD-11-0362	SHEET
DESIGNED BY EJH		DRAWN BY ML	DATE 8-20-2015	S2-02
CHECKED BY ML		APPROVED BY ML	NO. 52 = 102 SHEETS	



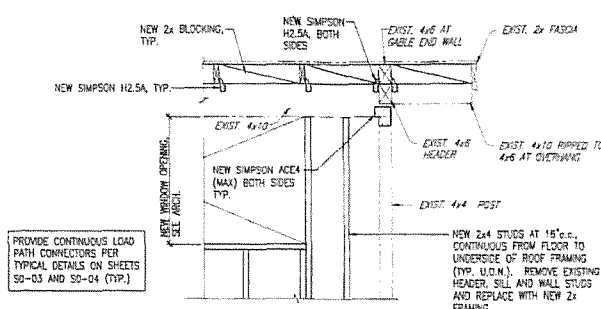


REVISION NO.	DATE	DESCRIPTION	BY	DATE

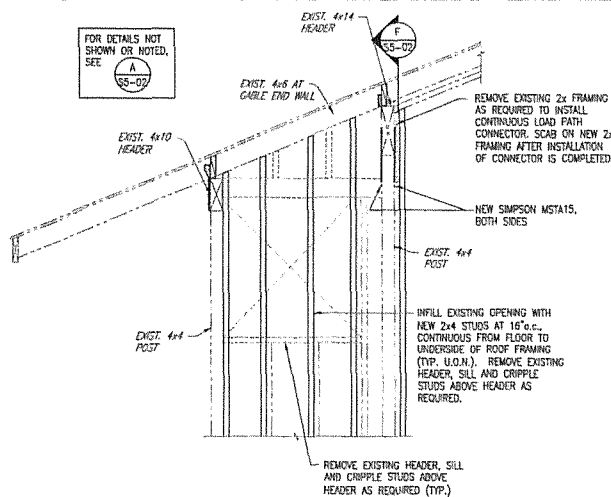
UNIVERSITY OF HAWAII STATE OF HAWAII UNIVERSITY OF HAWAII COLLEGE LAKE PROJECT NUMBER 20-11-5362 2245A & 2245B HOSPITALITY ACADEMY RENOVATION KAHULU, HAWAII		SS-01
ROOF SECTIONS AND DETAILS		
DESIGNED BY SLSH & ASSOC., INC. 1706 YOUNG STREET, 2ND FLOOR HONOLULU, HAWAII 96813	DRAWN BY J.C.	CHECKED BY J.C.
DATE 8-20-2013	APPROVED BY J.C.	SHEET NO. 123



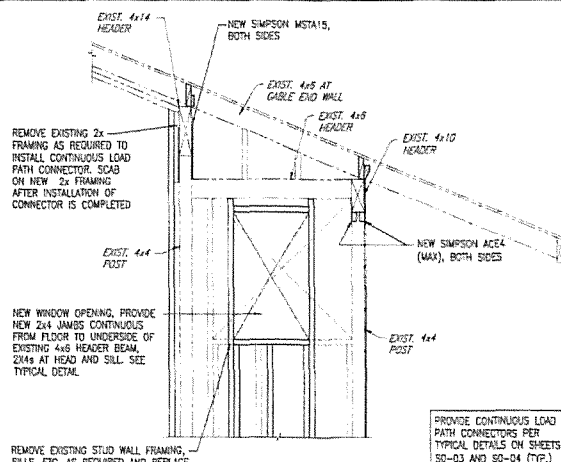
A SECTION
SS-02 SCALE: 3/4" = 1'-0"



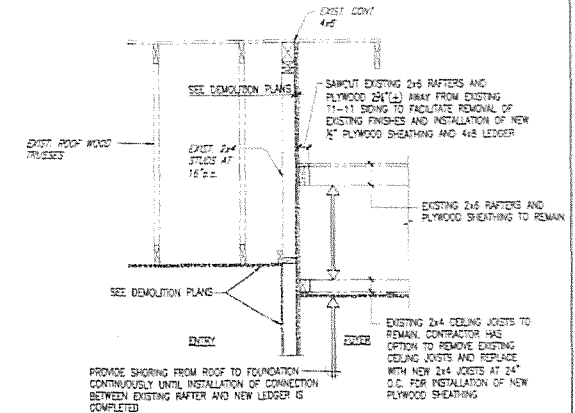
B SECTION
SS-02 SCALE: 3/4" = 1'-0"



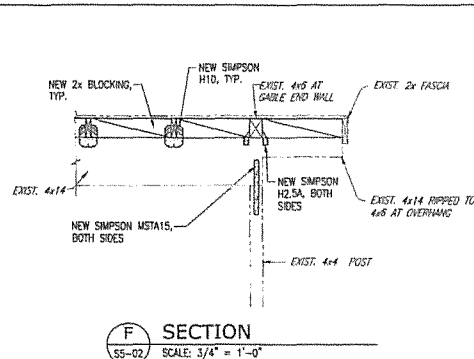
C SECTION
SS-02 SCALE: 3/4" = 1'-0"



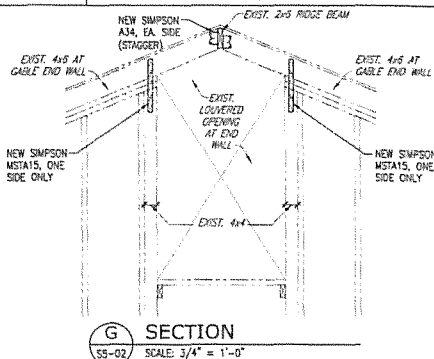
D SECTION
SS-02 SCALE: 3/4" = 1'-0"



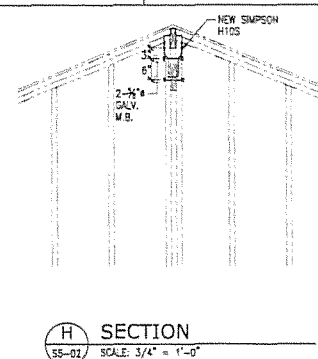
E SECTION
SS-02 SCALE: 3/4" = 1'-0"



F SECTION
SS-02 SCALE: 3/4" = 1'-0"



G SECTION
SS-02 SCALE: 3/4" = 1'-0"



H SECTION
SS-02 SCALE: 3/4" = 1'-0"

REVISION	DATE	DESCRIPTION	BY	DATE
1				
2				
3				
4				
5				
6				
7				
8				
9				
10				
11				
12				
13				
14				
15				
16				
17				
18				
19				
20				
21				
22				
23				
24				
25				
26				
27				
28				
29				
30				
31				
32				
33				
34				
35				
36				
37				
38				
39				
40				
41				
42				
43				
44				
45				
46				
47				
48				
49				
50				
51				
52				
53				
54				
55				
56				
57				
58				
59				
60				
61				
62				
63				
64				
65				
66				
67				
68				
69				
70				
71				
72				
73				
74				
75				
76				
77				
78				
79				
80				
81				
82				
83				
84				
85				
86				
87				
88				
89				
90				
91				
92				
93				
94				
95				
96				
97				
98				
99				
100				